

隔減震應用之介紹

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$$\frac{1}{2} M (\dot{X}_t)^2 + \int C \dot{X} dX + \int KX dX = \int M \ddot{X}_g dX_g$$

Equation of Motion

$$M\ddot{X} + C\dot{X} + KX = -M\ddot{X}_g$$

Absolute Energy Equation

$$\frac{1}{2} M (\dot{X}_t)^2 + \int C \dot{X} dX + \int KX dX = \int M \ddot{X}_t dX_g$$

Absolute Energy Equation of A Structure Responding to An Earthquake

Energy Demand by Earthquake

Energy Supply of Structure

$$E_I = E_k + E_s + E_h + E_d$$

Earthquake Input Energy

Kinetic Energy

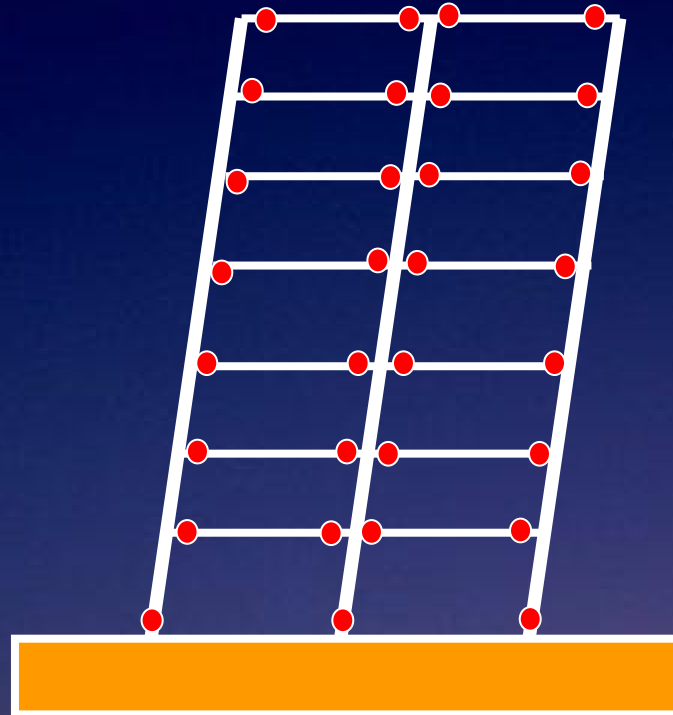
Elastic Strain Energy

Hysteretic Energy

Viscous Damping Energy

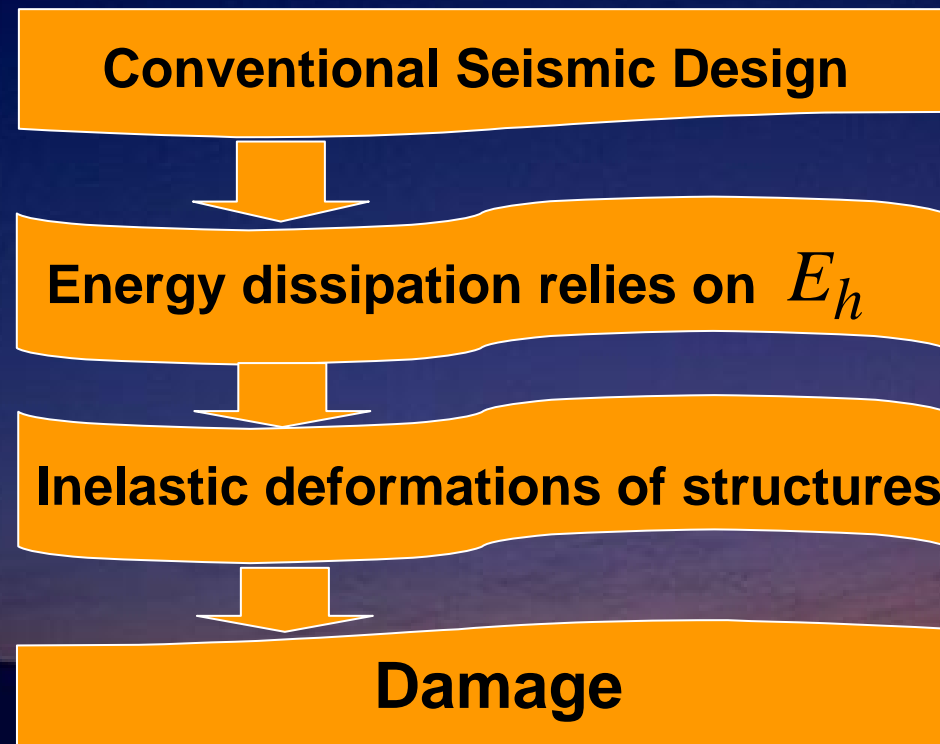
$$\frac{1}{2} M (\dot{X}_t)^2 + \int C \dot{X} dX + \int KX dX = \int M \ddot{X}_t dX_g$$

Ideal Mechanism for Conventional Structures



Energy Consideration

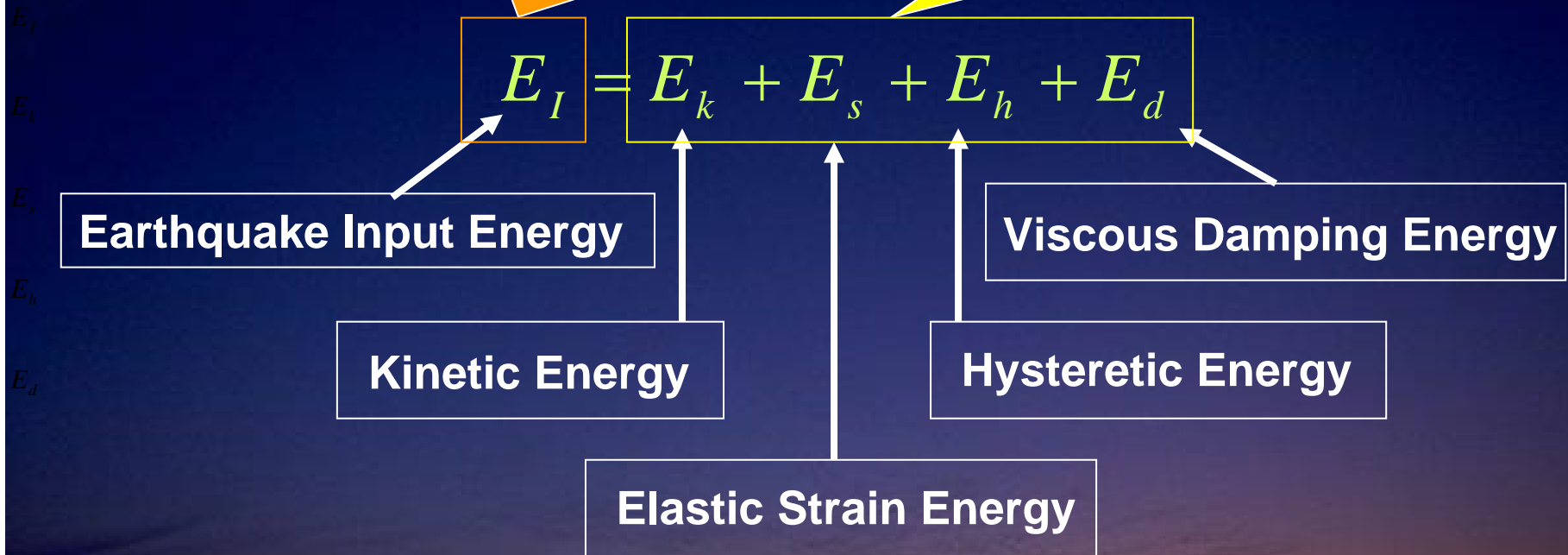
$$E_I = E_k + E_s + E_h + E_d$$



Absolute Energy Equation of A Structure Responding to An Earthquake

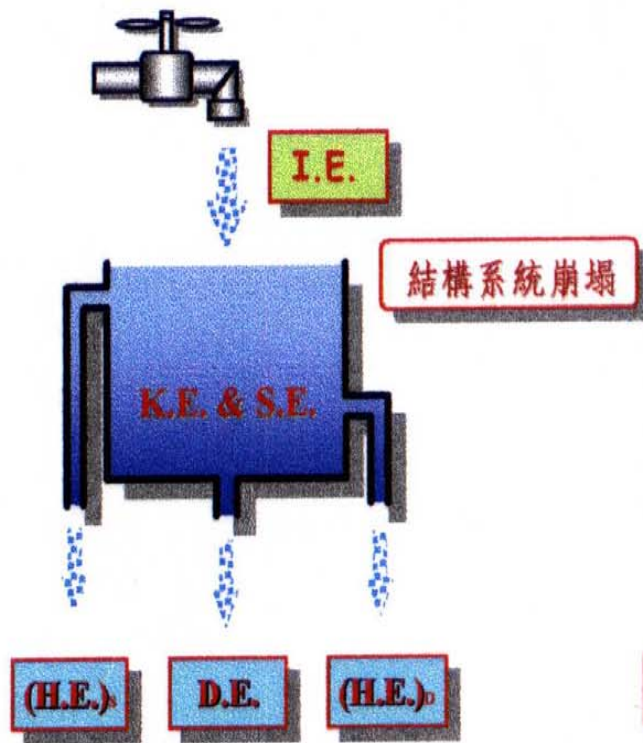
Energy Demand by Earthquake

Energy Supply of Structure



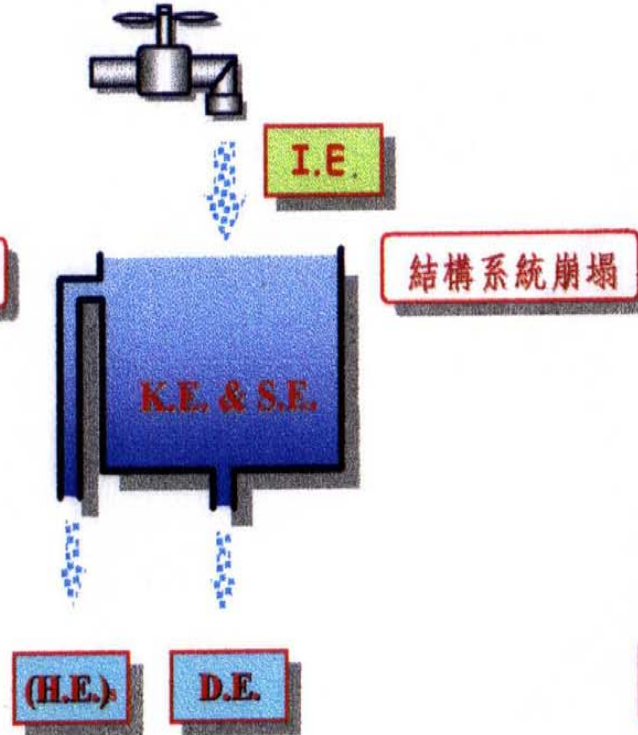
$$\frac{1}{2} M (\dot{X}_t)^2 + \int C \dot{X} dX + \int K X dX = \int M \ddot{X}_t dX_g$$

增加阻尼結構系統



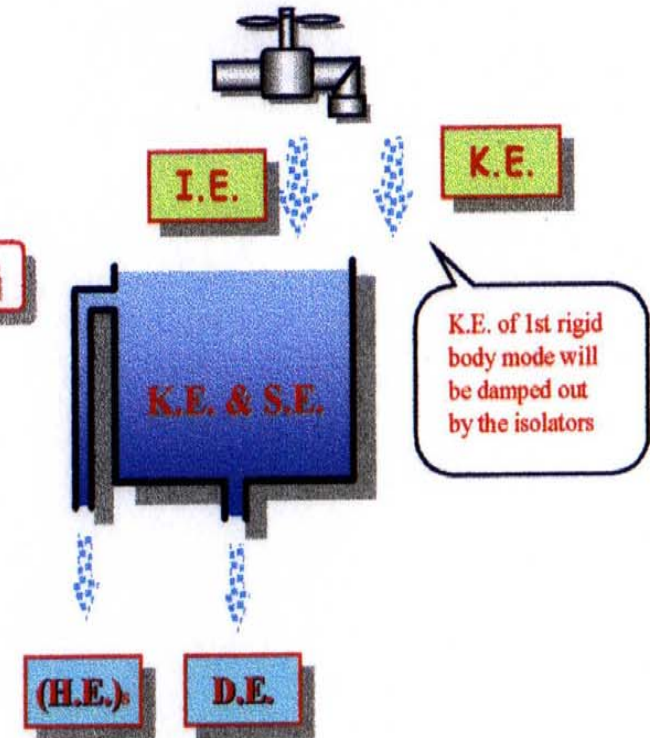
容器行為模式(一)
增加阻尼結構

一般結構系統



容器行為模式(二)
一般結構

隔震結構系統



容器行為模式(三)
隔震結構

I.E. : Seismic Input Energy

K.E. : Kinetic Energy

S.E. : Elastic Strain Energy

D.E. : Viscous damping Energy

(H.E.)s : Hysteresis Energy Dissipated by Structural Elements

(H.E.)_D : Hysteresis Energy Dissipated by Energy Dissipators (Dampers)

Strategies for Seismic Protection of Structures

- (1) Increase Energy Dissipation Capacity of Structures
 - Implementation of Energy Dissipation Devices
- (2) Reduce Energy Demand by Earthquake
 - Adoption of Seismic Isolation Design

Energy Dissipation Devices Applied In Taiwan

(1) Hysteresis Type Dampers: (Hysteretic Energy)

ADAS (Added Damping and Stiffness)

TADAS (Triangular ADAS)

LYSSP (Low Yield Steel Shear Panel)

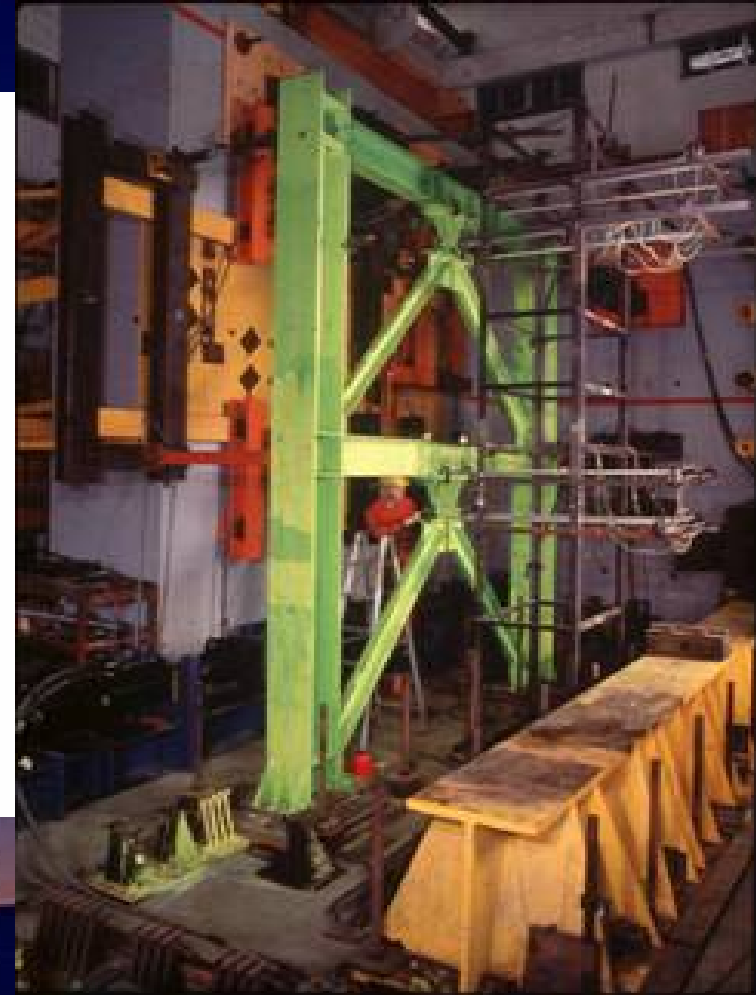
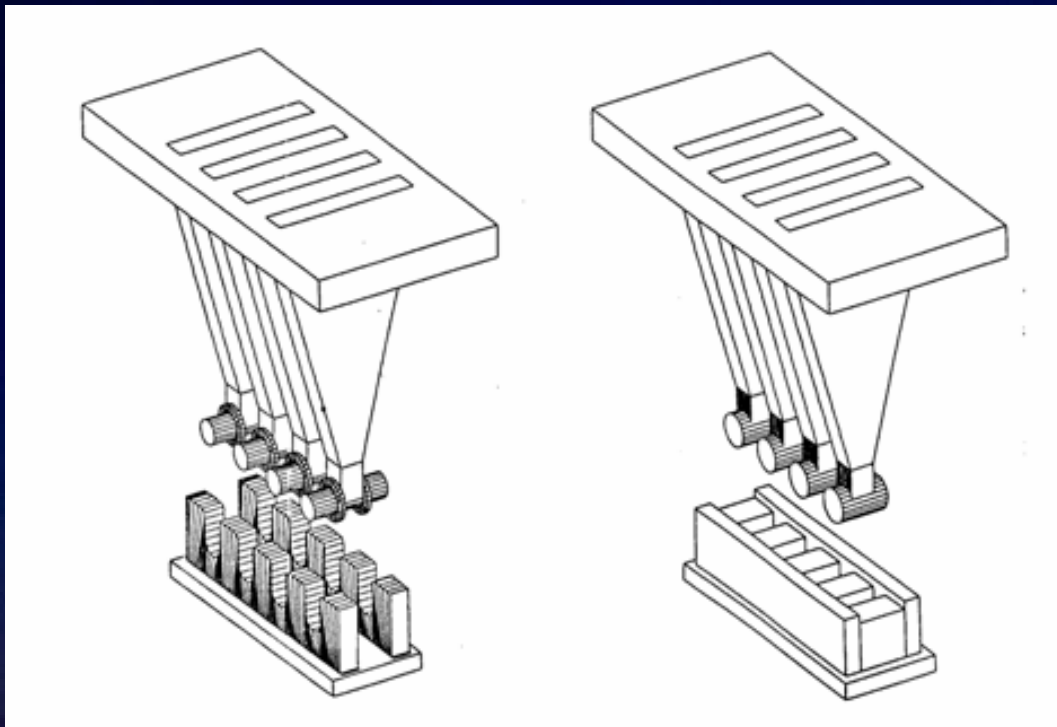
(2) Velocity-Dependent Damper: (Viscous Damping Energy)

Visco-Elastic Dampers, Viscous Dampers

(3) Improved Energy Dissipation Design: (Hysteretic Energy)

BRB (Buckling Restrained Braces or Unbonded Braces)

Hysteretic Type Dampers -- TADAS



Experimental Investigation

京華城 Taipei Living Mall



TADAS Installation



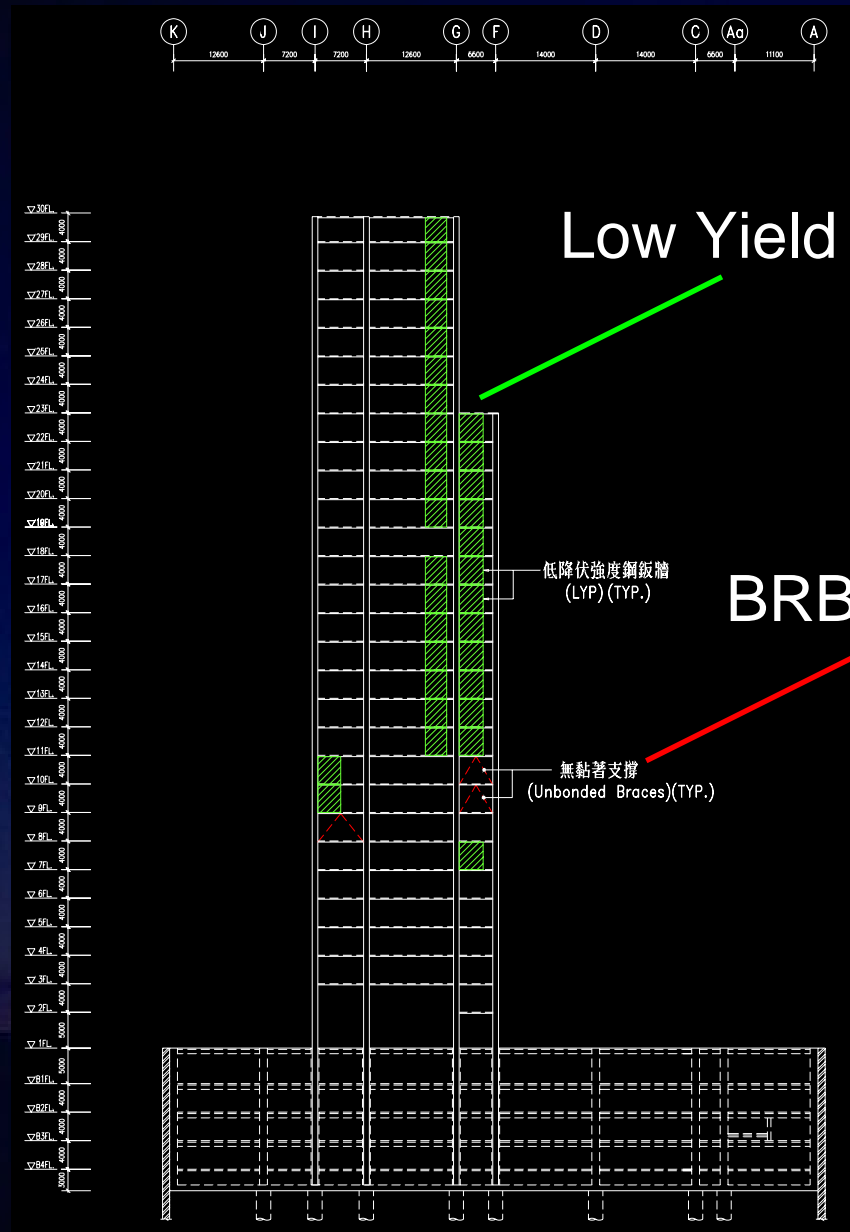
Taipei County Hall

33 story steel structure

Designed before Chi-Chi

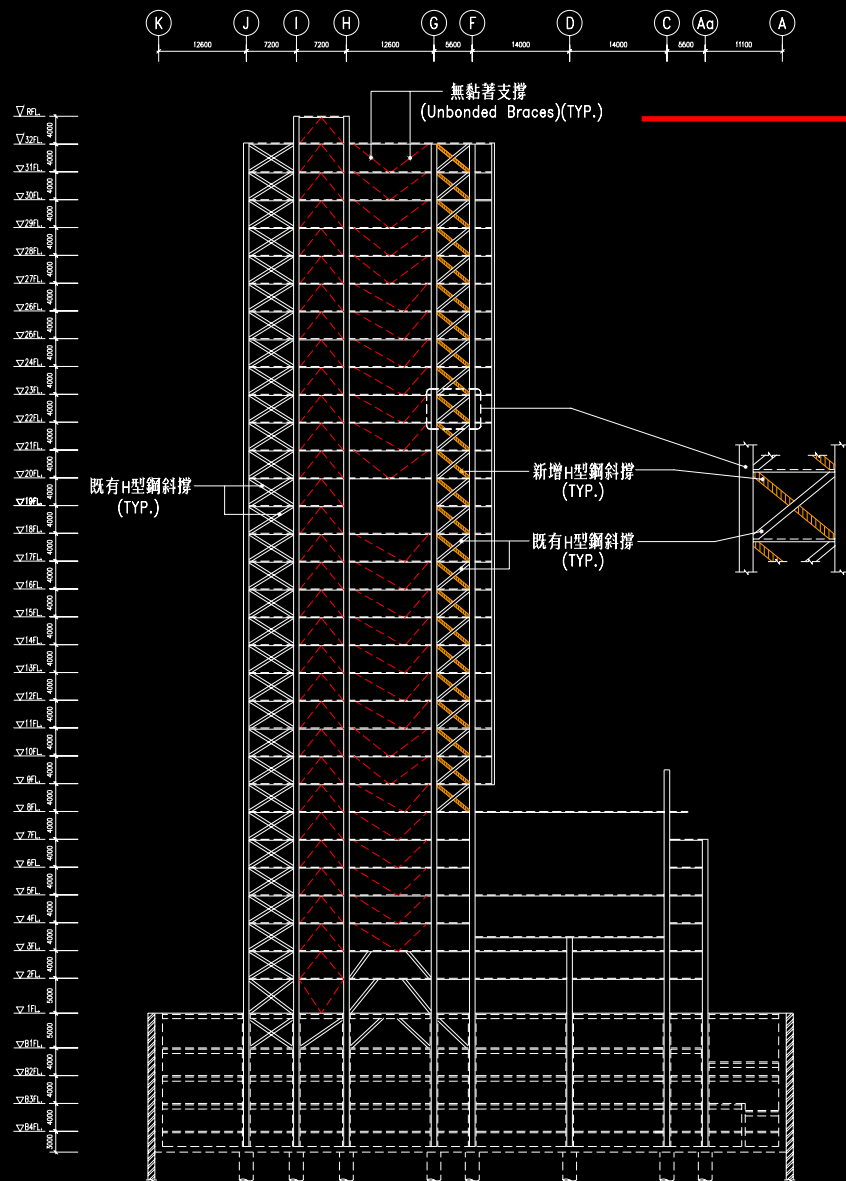
Upgraded after Chi-Chi



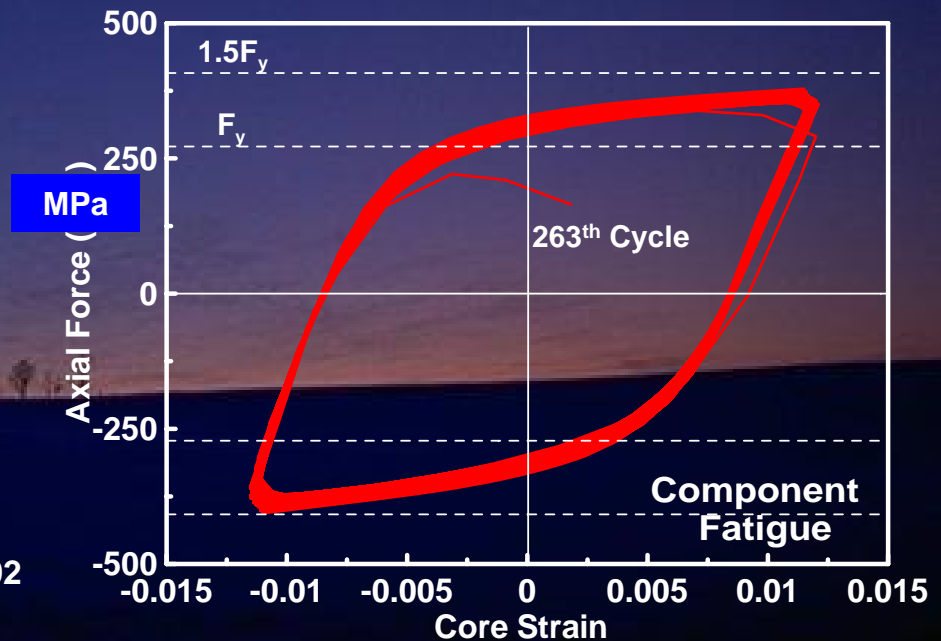
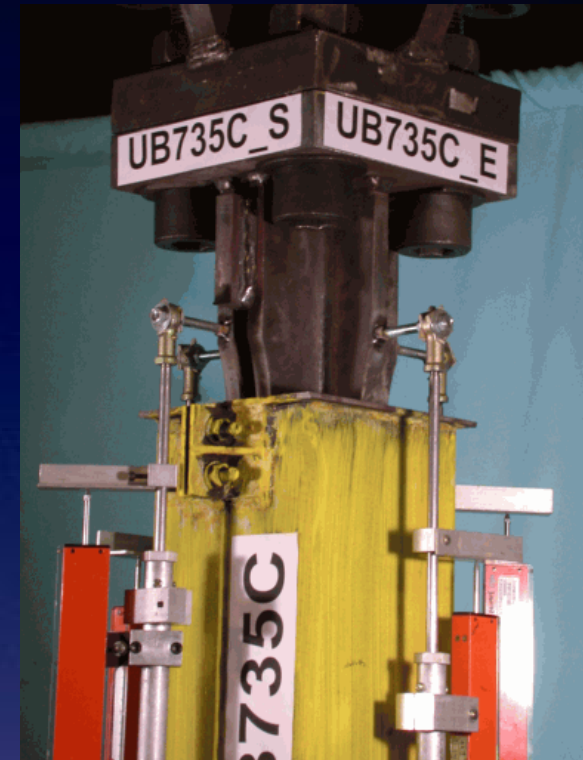
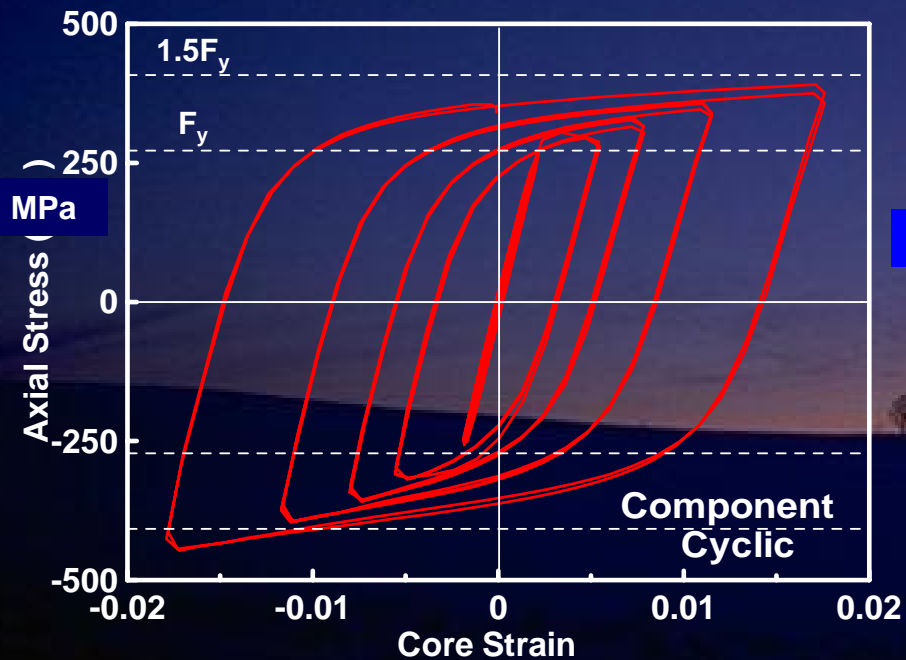
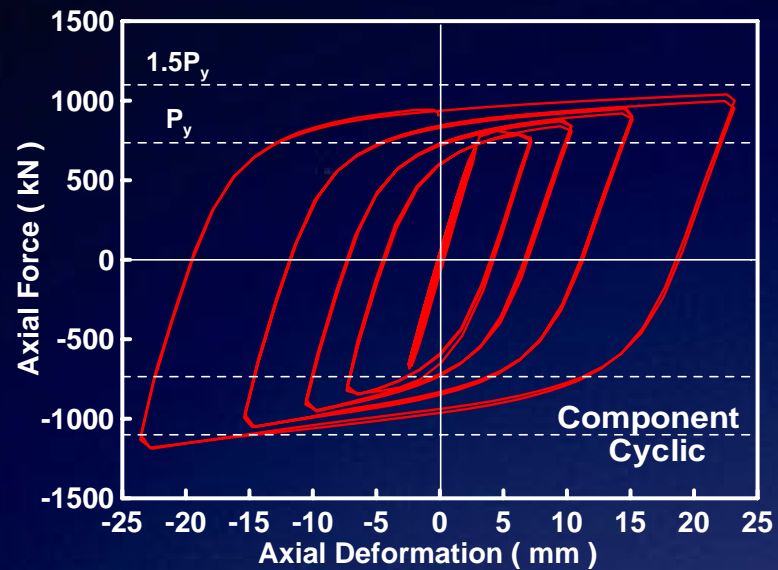


Low Yield Steel Panel

BRB (Buckling Restrained Braces)

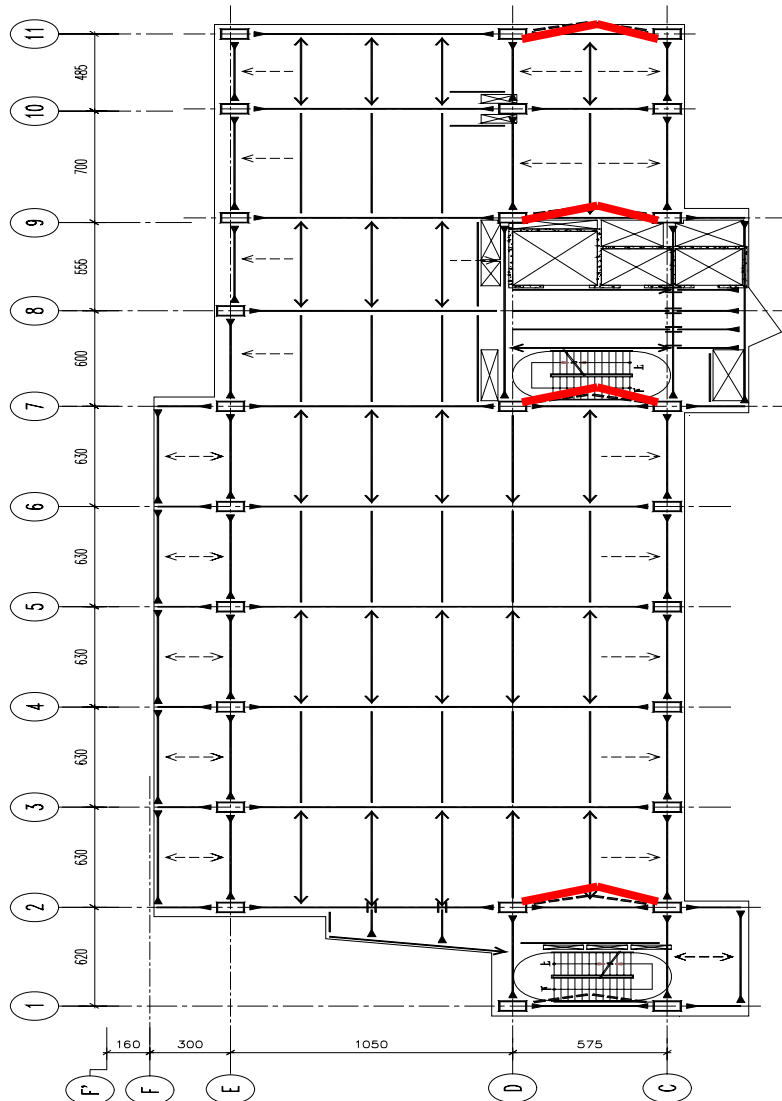


BRB Test Results

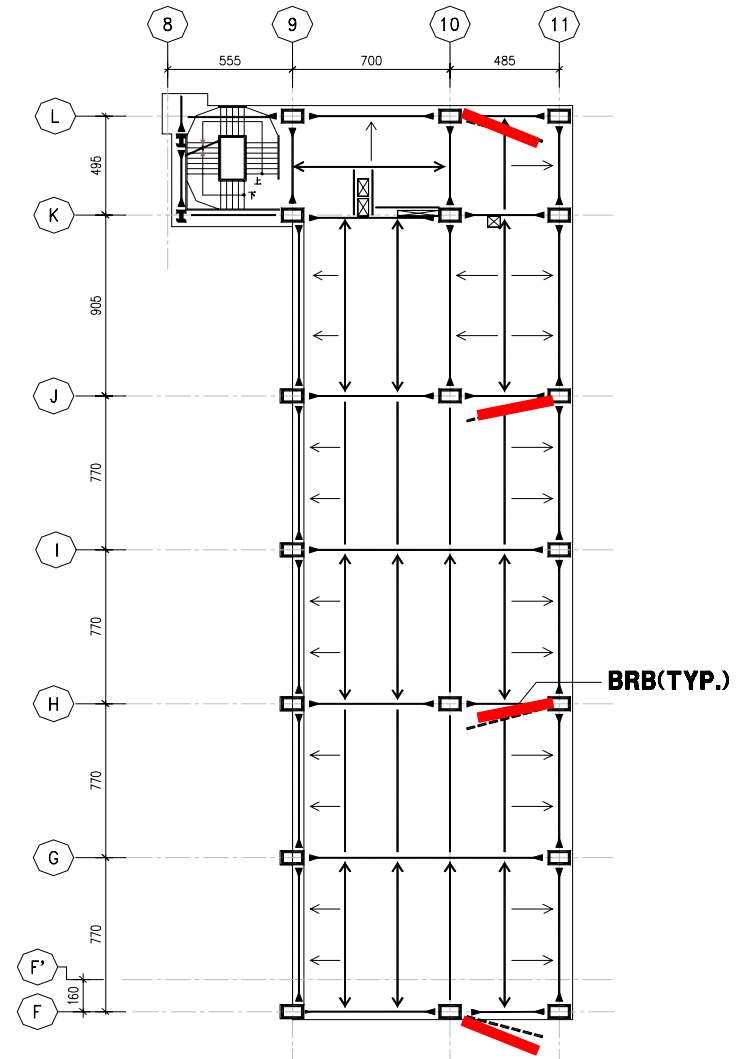




West Wing LYSSP



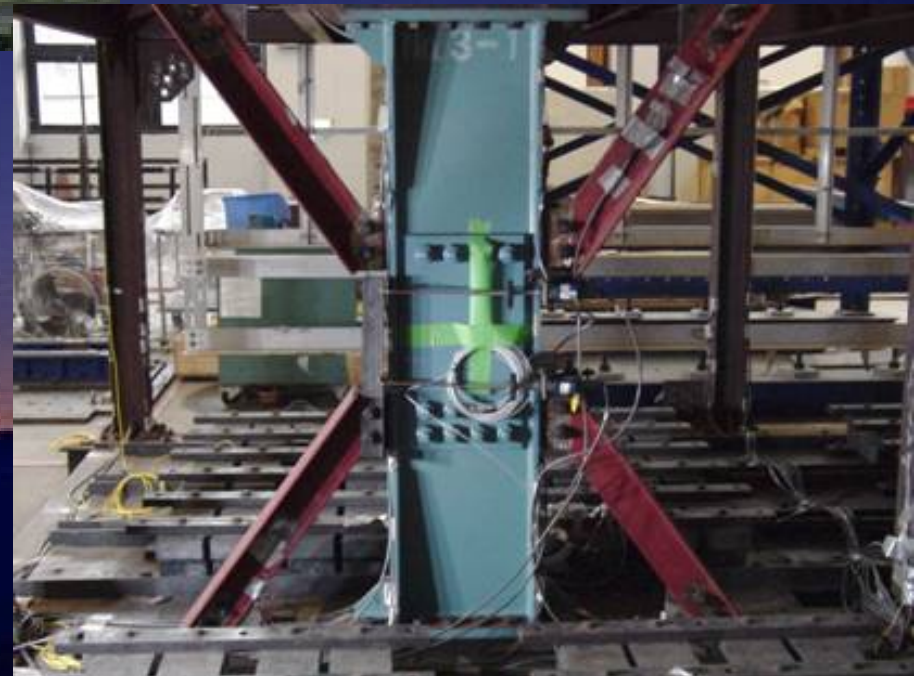
South Wing BRB





宏盛帝寶
Taipei Treasure Palace

Viscoelastic Damper



現場結構C.F.T.與VEM-Damper已施工至七樓
（實景拍攝日期 2002.12.18）

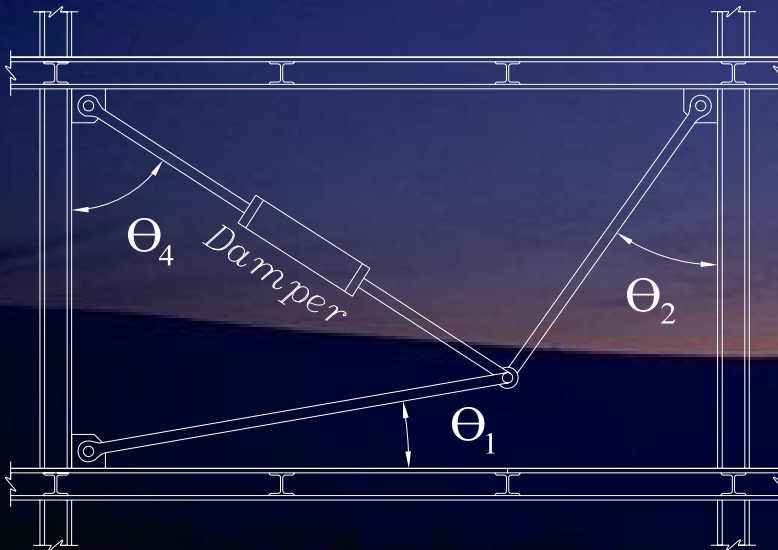
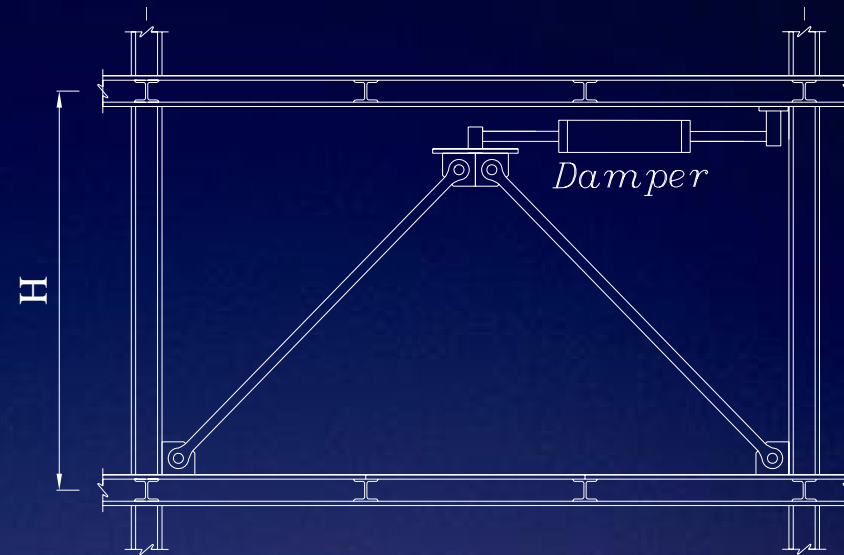
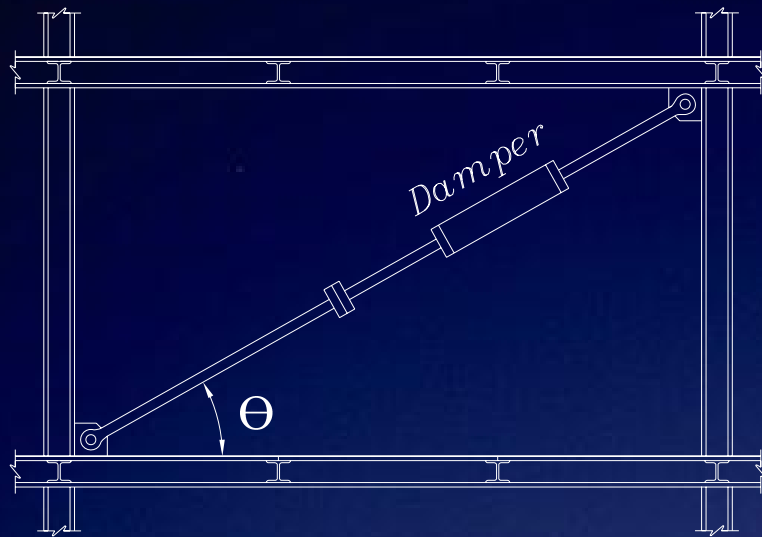
Visco-Elastic Shear Wall

帝寶

傳世至寶
Transit Palace



Typical Installation Scheme of Viscous Dampers



Viscous Dampers

Tai-Shin Bank





Tai-Shin Bank Headquarter

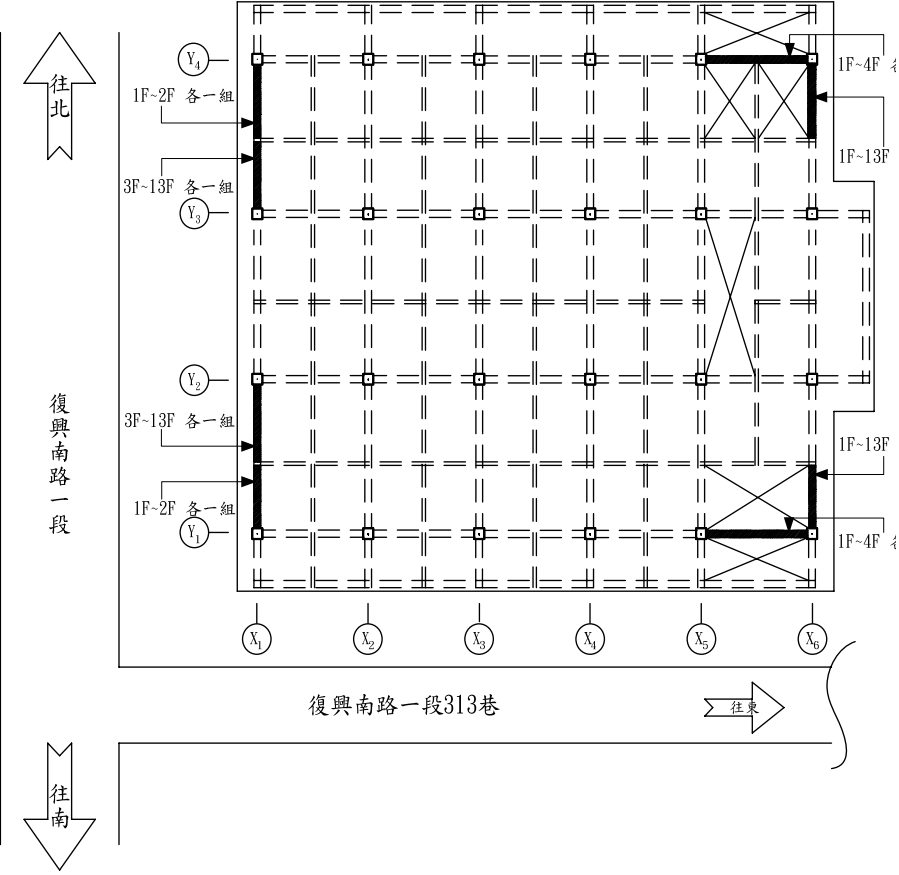




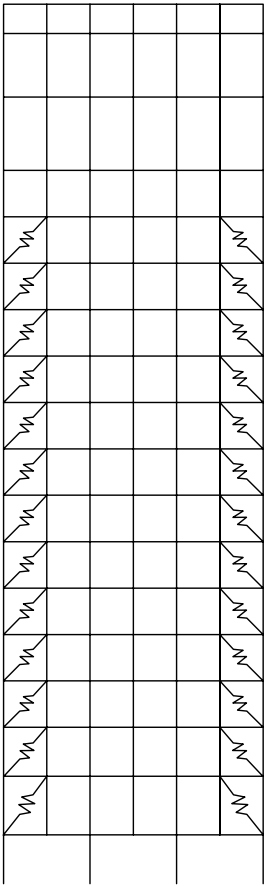
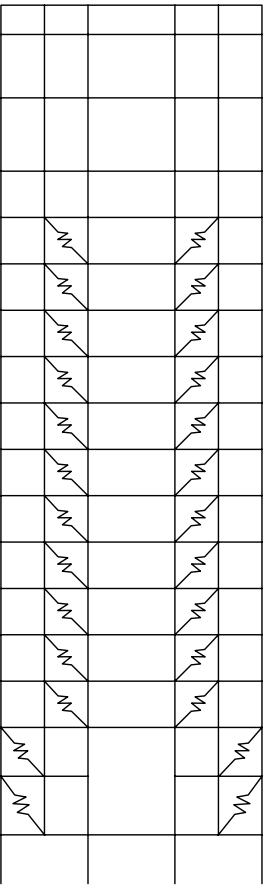
Headquarter of Buddhist Association



Plan



Elevation



Force-Velocity Relationship of Viscous Dampers

$$F_D = C |\dot{u}|^\alpha \operatorname{sgn}(\dot{u})$$

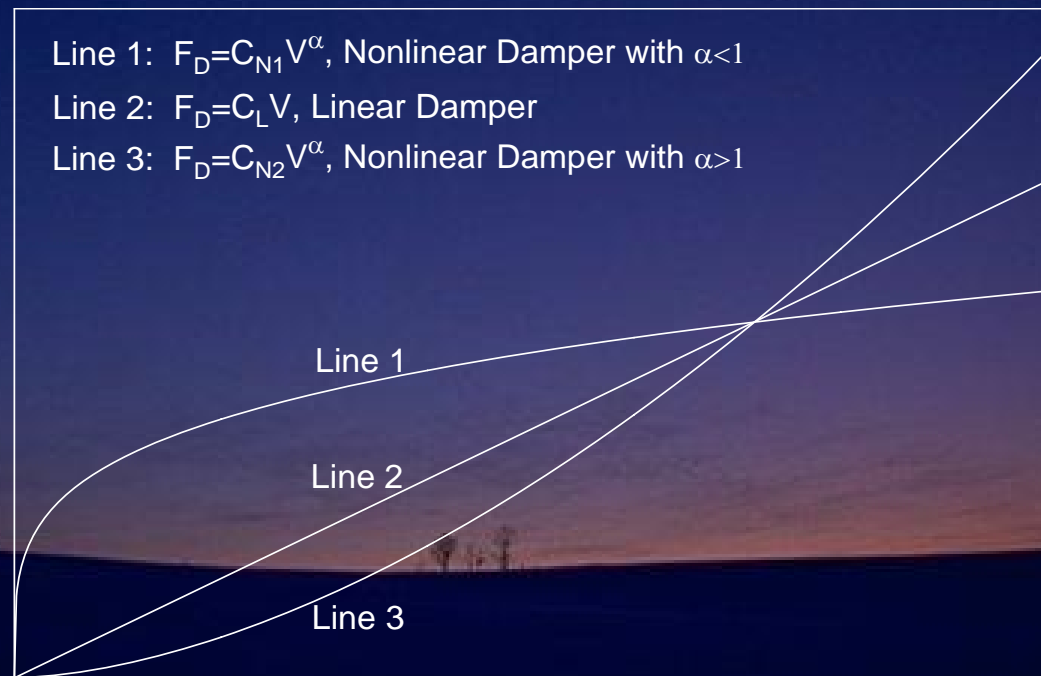
$$\alpha = 1$$

Linear Viscous Dampers

$$0 < \alpha < 1$$

Nonlinear Viscous Dampers

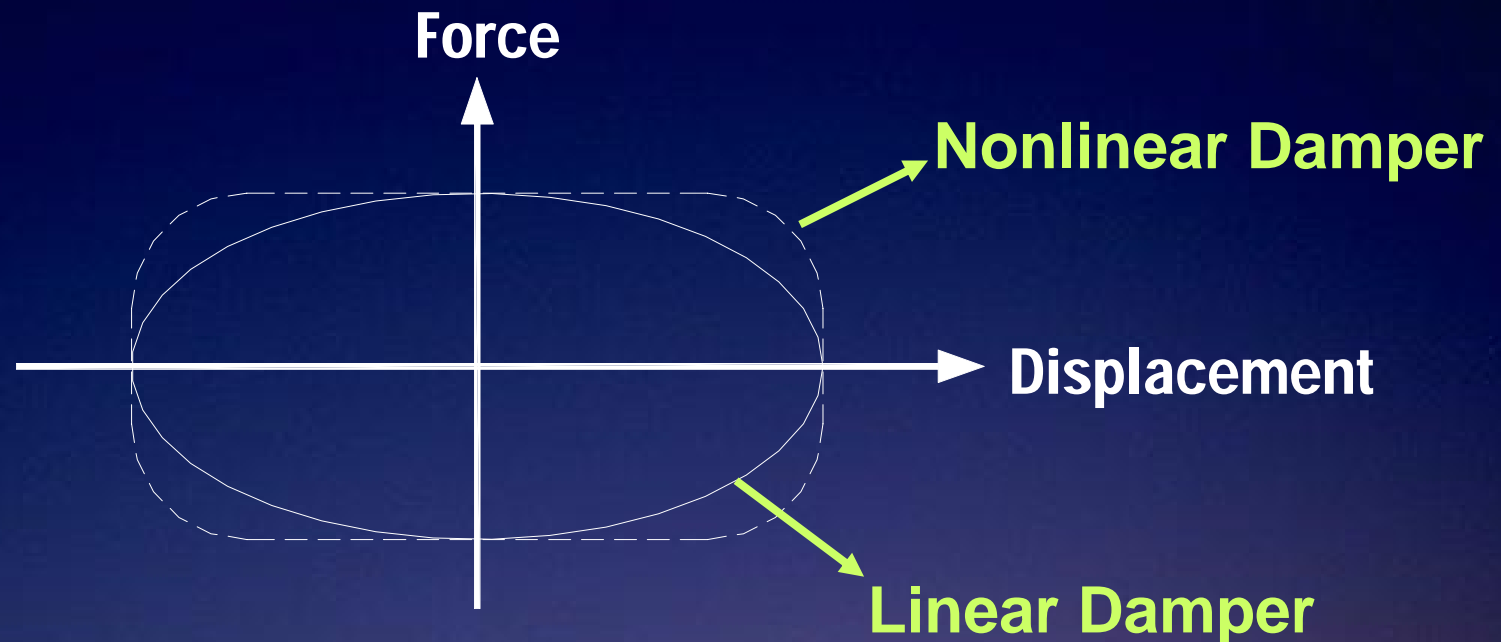
Damper Force, F_D



Velocity, V

Mechanical Properties of Fluid Viscous Dampers

Hysteresis Loops of Viscous Dampers (Sinusoidal Motion)



No storage stiffness!

Viscous dampers won't change the natural frequency of the primary structure.

Mechanical Properties of Fluid Viscous Dampers

Damping Ratio Contributed by Nonlinear Viscous Dampers

Diagonal Brace $u_j = u_i \cos \theta_j = A \phi_{rj} \cos \theta_j$

$$\xi_{eff} = \xi_0 + \frac{\sum_j \lambda_j C_j \phi_{rj}^{1+\alpha} \cos^{1+\alpha} \theta_j}{2\pi A^{1-\alpha} \omega^{2-\alpha} \sum_i m_i \phi_i^2}$$

K-Brace

$$\theta = 0$$

$$\xi_{eff} = \xi_0 + \frac{\sum_j \lambda_j C_j \phi_{rj}^{1+\alpha}}{2\pi A^{1-\alpha} \omega^{2-\alpha} \sum_i m_i \phi_i^2}$$

Grand Palace of Taipei



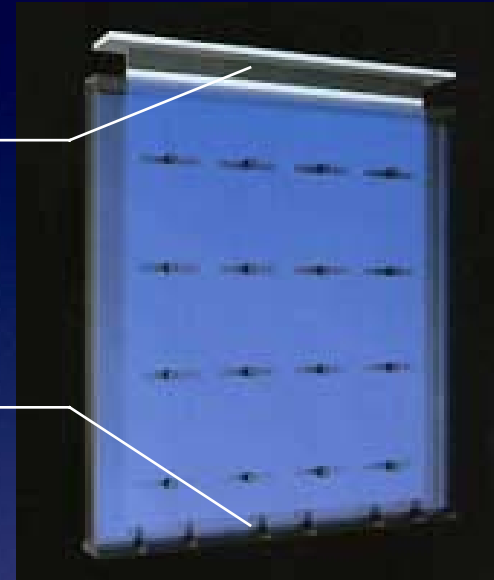
Taipei 101 Tower

Taipei City Hall



中間層鋼板連接
上部抗彎構架

外部鋼板連接
下部抗彎構架



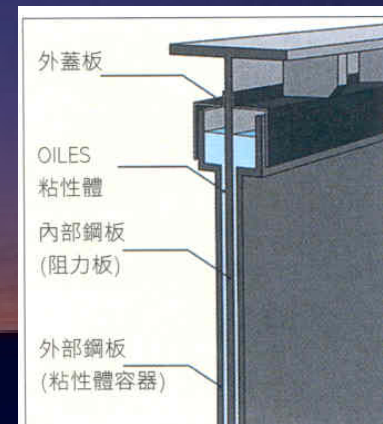
基本構造圖



**Viscous
Material**



Damper
in Place



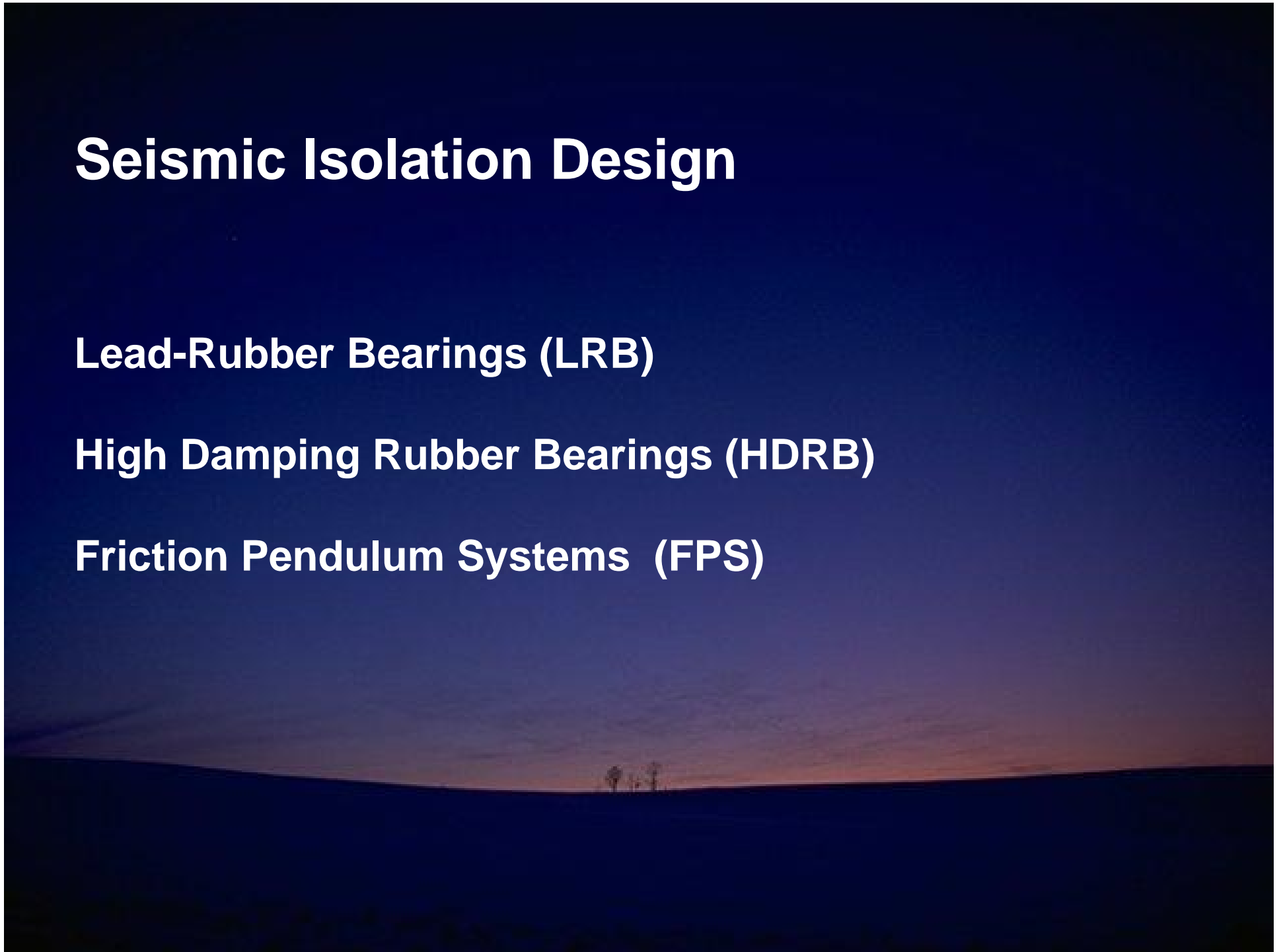
Sectional View

Seismic Isolation Design

Lead-Rubber Bearings (LRB)

High Damping Rubber Bearings (HDRB)

Friction Pendulum Systems (FPS)



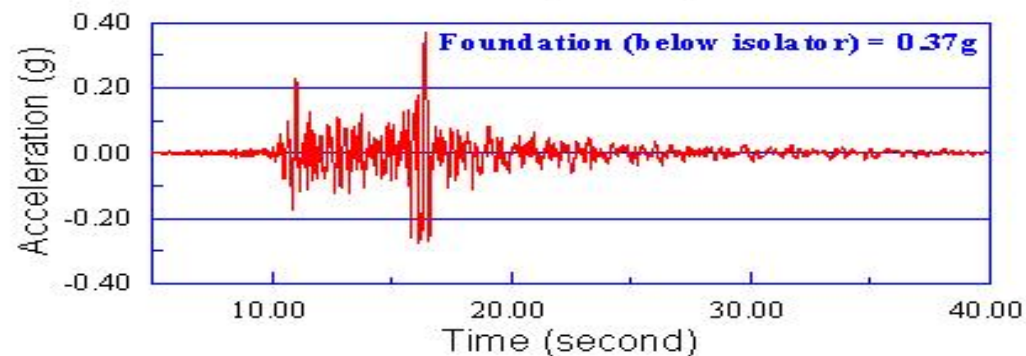
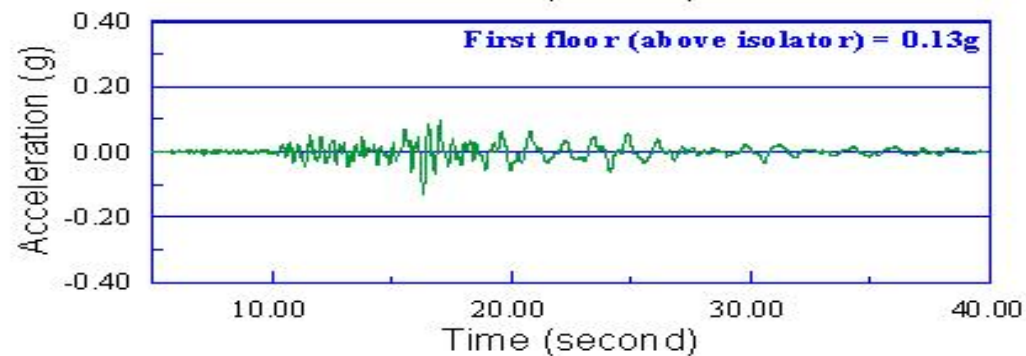
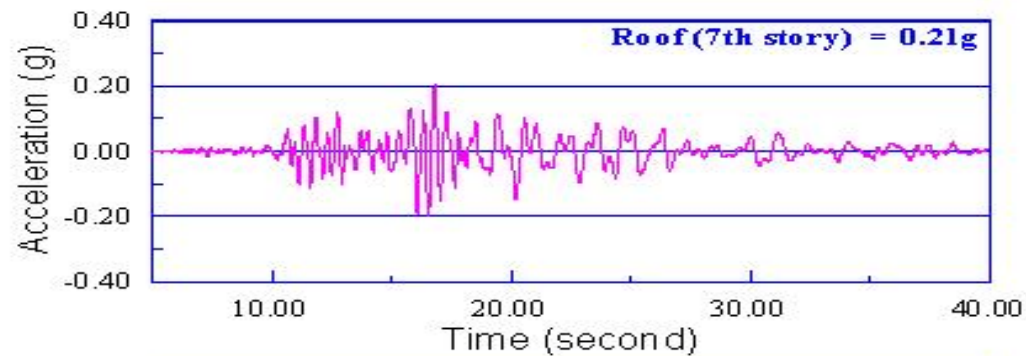
USC Hospital : 1994 Northridge Earthquake

81 Rubber Bearings

68 Lead-Rubber Bearings



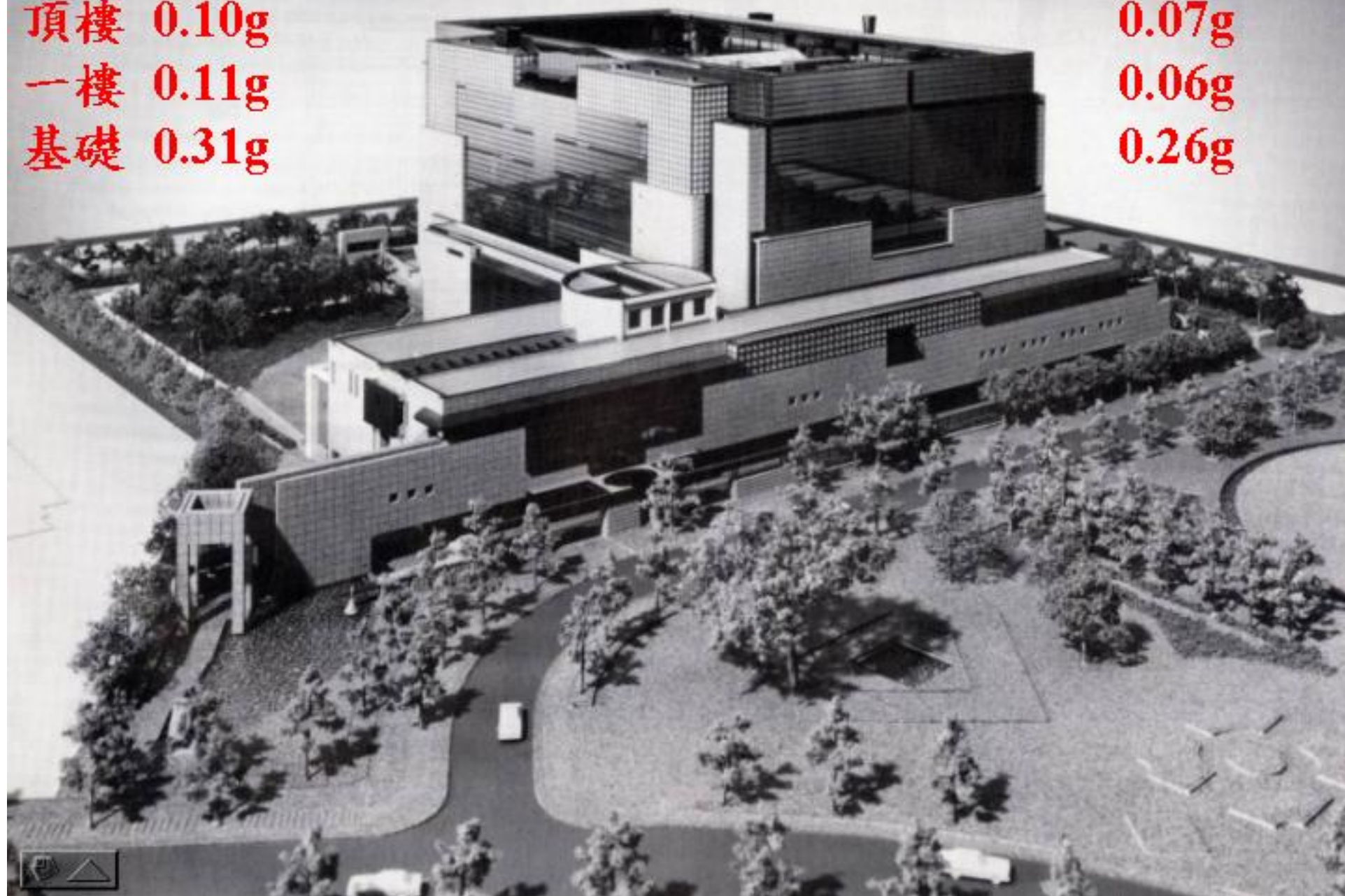
MEASURED SEISMIC RESPONSES OF UNIV. OF SOUTHERN CALIFORNIA HOSPITAL DURING 1994 NORTHRIDGE EARTHQUAKE



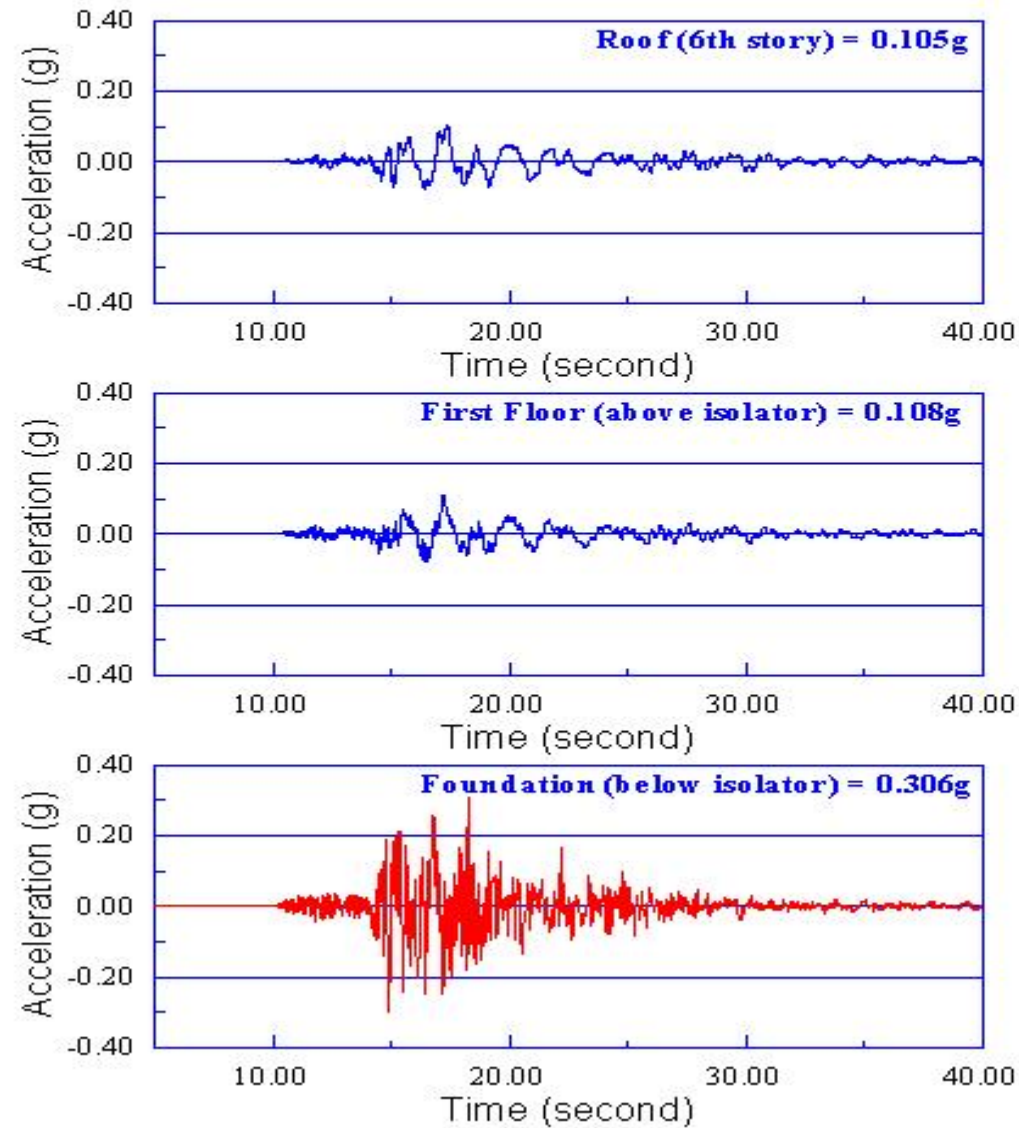
Japan Ministry of Post and Telecommunications

頂樓 0.10g
一樓 0.11g
基礎 0.31g

0.07g
0.06g
0.26g



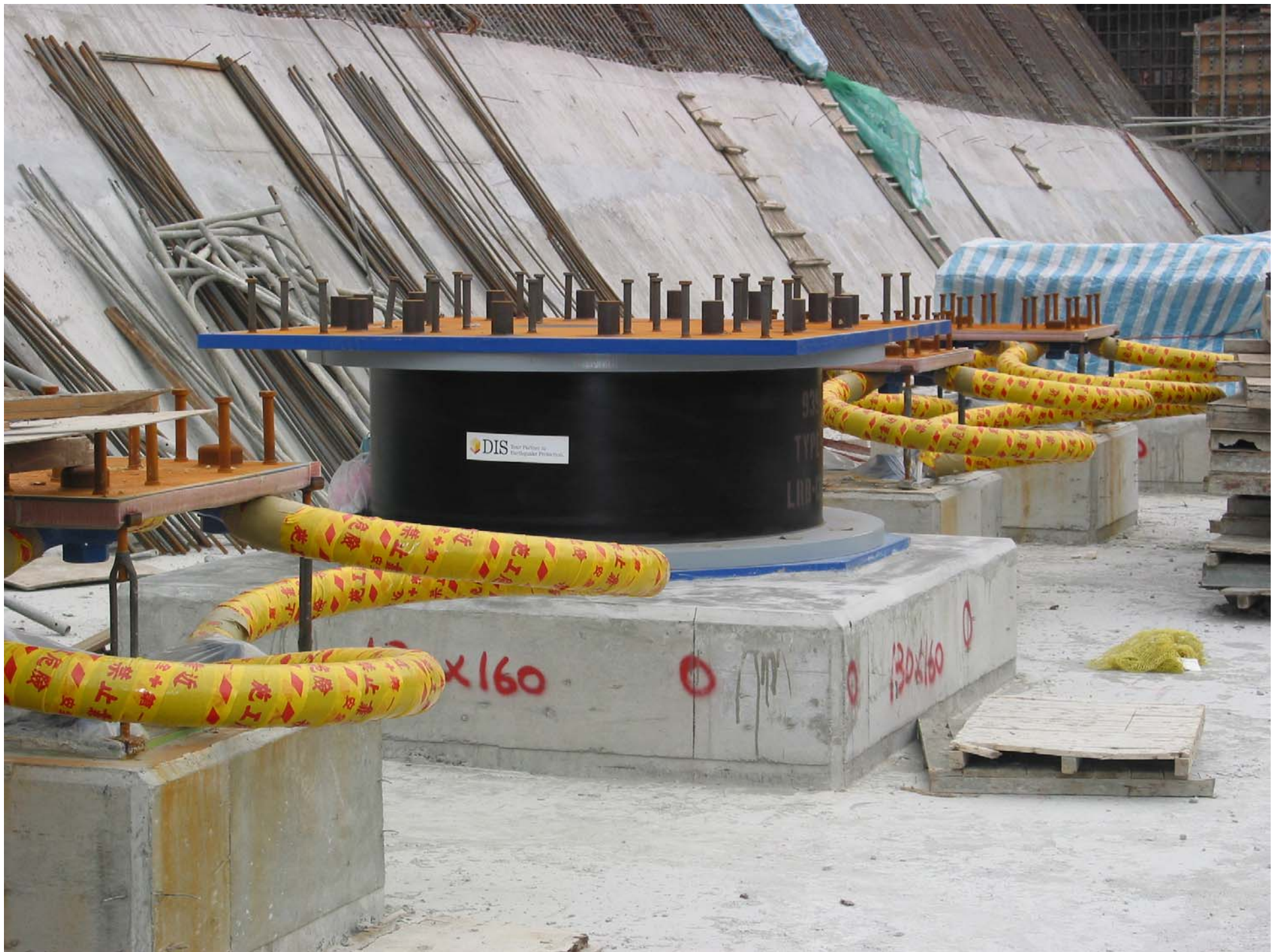
MEASURED SEISMIC RESPONSES OF THE COMPUTER CENTER OF JAPAN MINISTRY OF POST & TELECOMMUNICATIONS DURING 1995 KOBE EARTHQUAKE



Tzu-Chi Hospital at Taipei









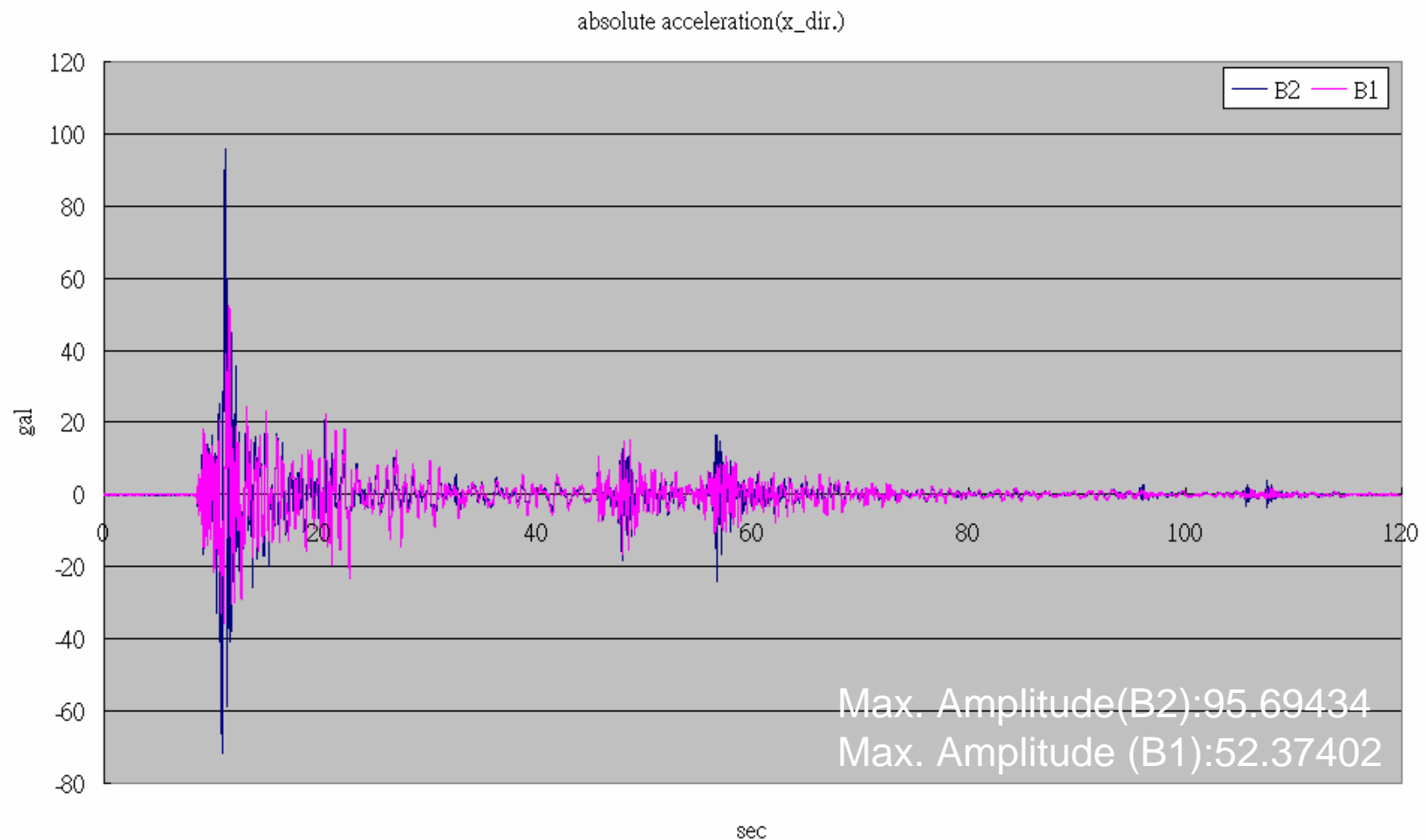


Trigger Time 2005/04/30 14:47:37.00

Sampling Freq. (Hz) 200

Duration Time (second) 120.000

Number of Data 24000

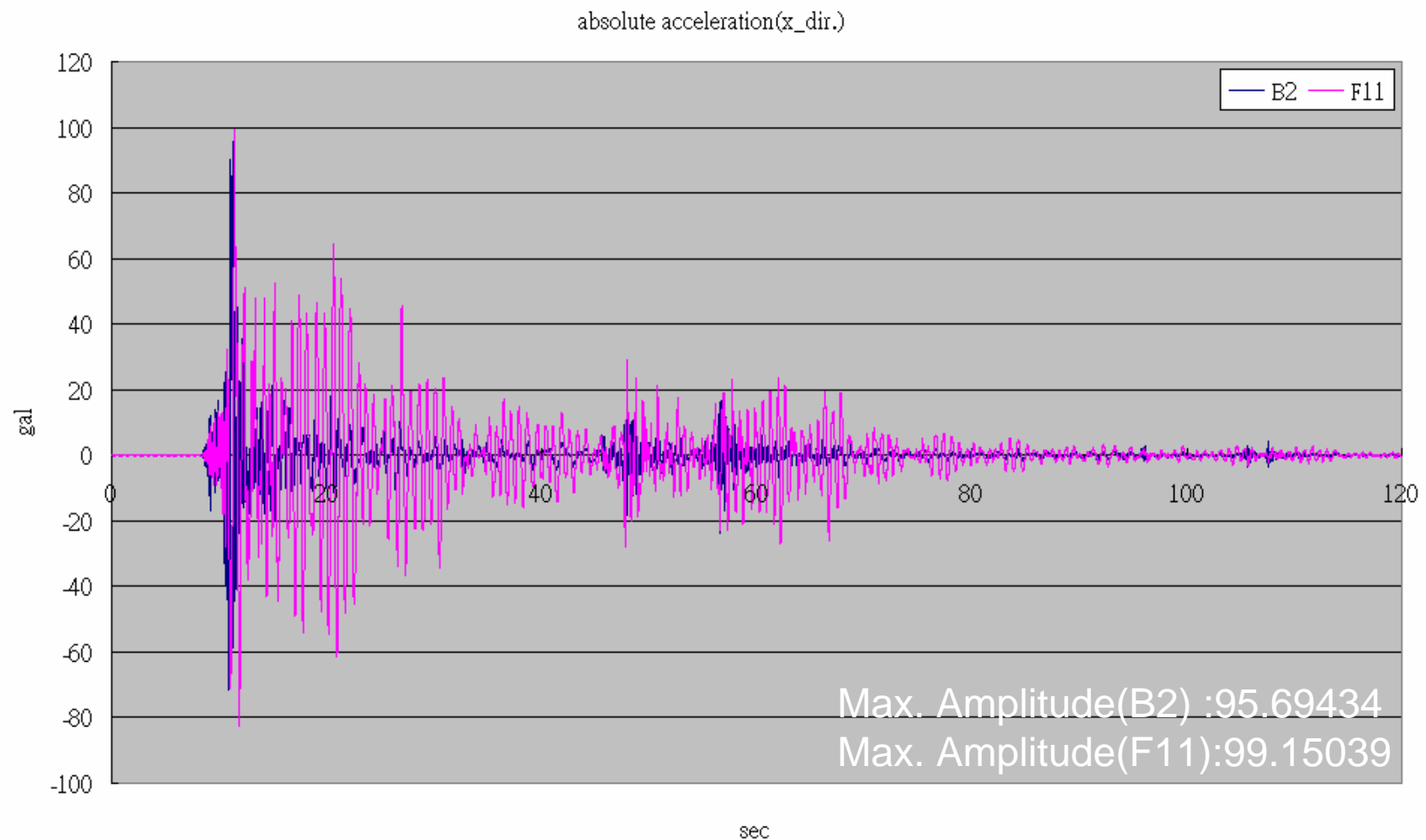


Trigger Time 2005/04/30 14:47:37.00

Sampling Freq. (Hz) 200

Duration Time (second) 120.000

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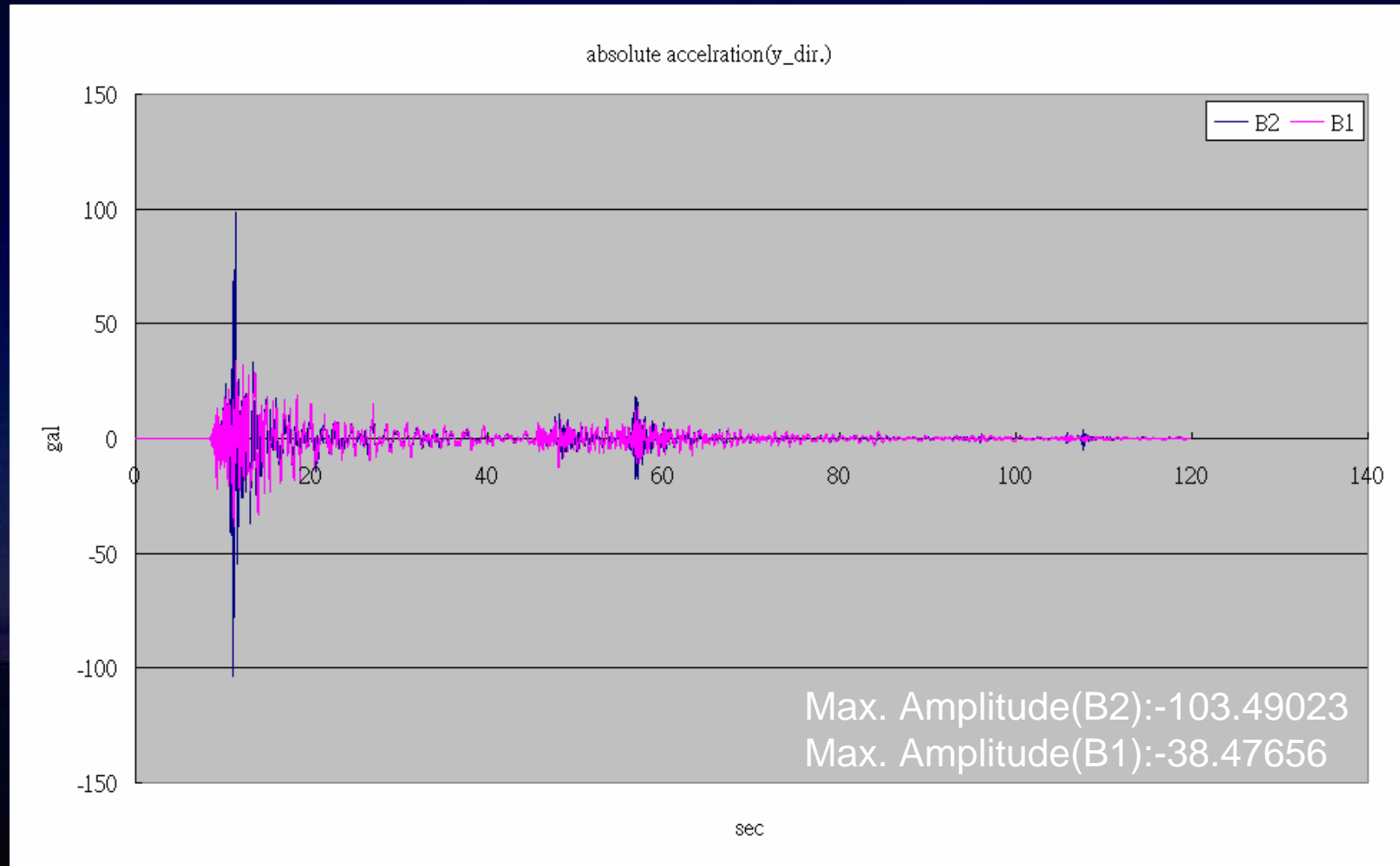


Trigger Time 2005/04/30 14:47:37.00

Sampling Freq. (Hz) 200

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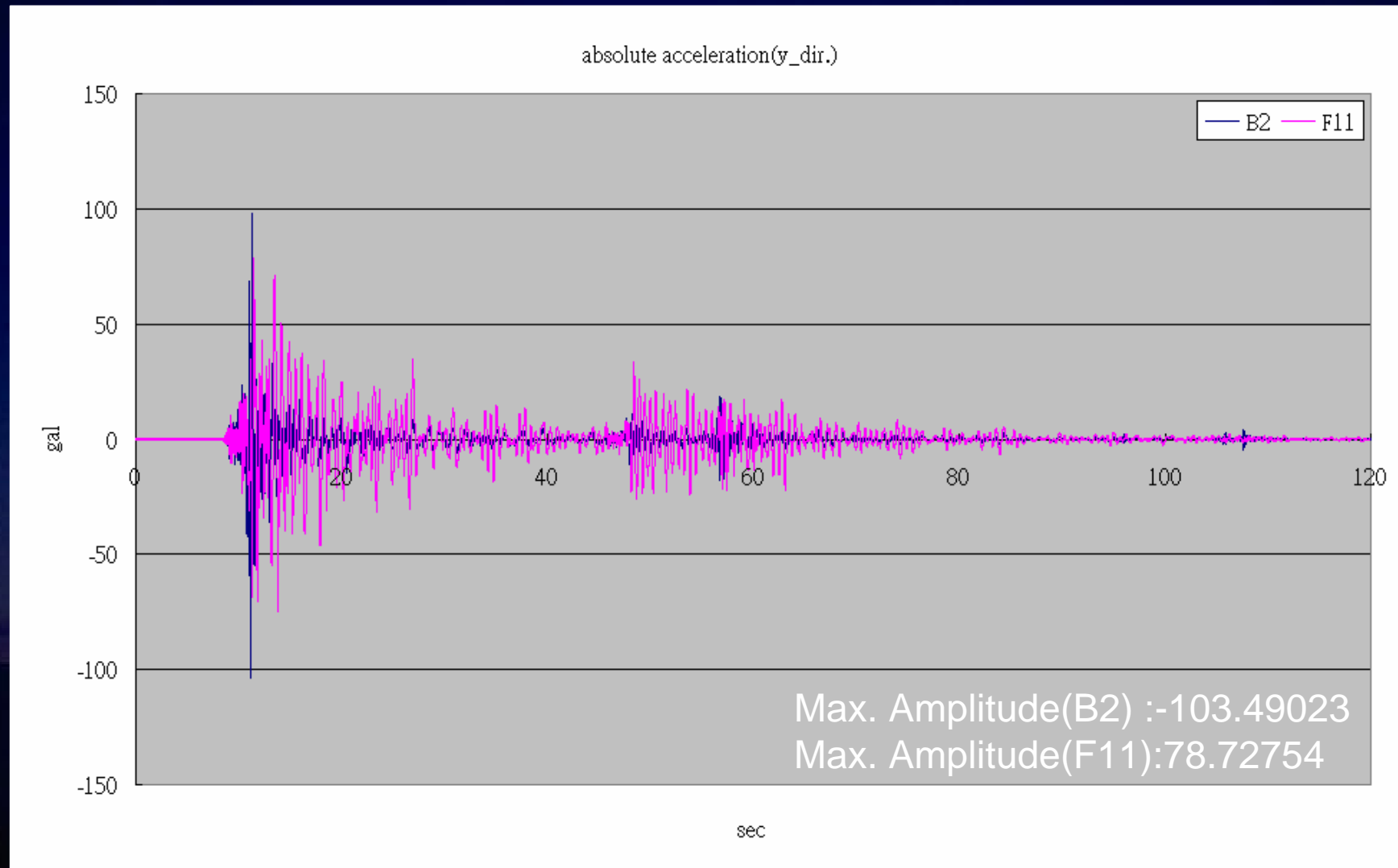


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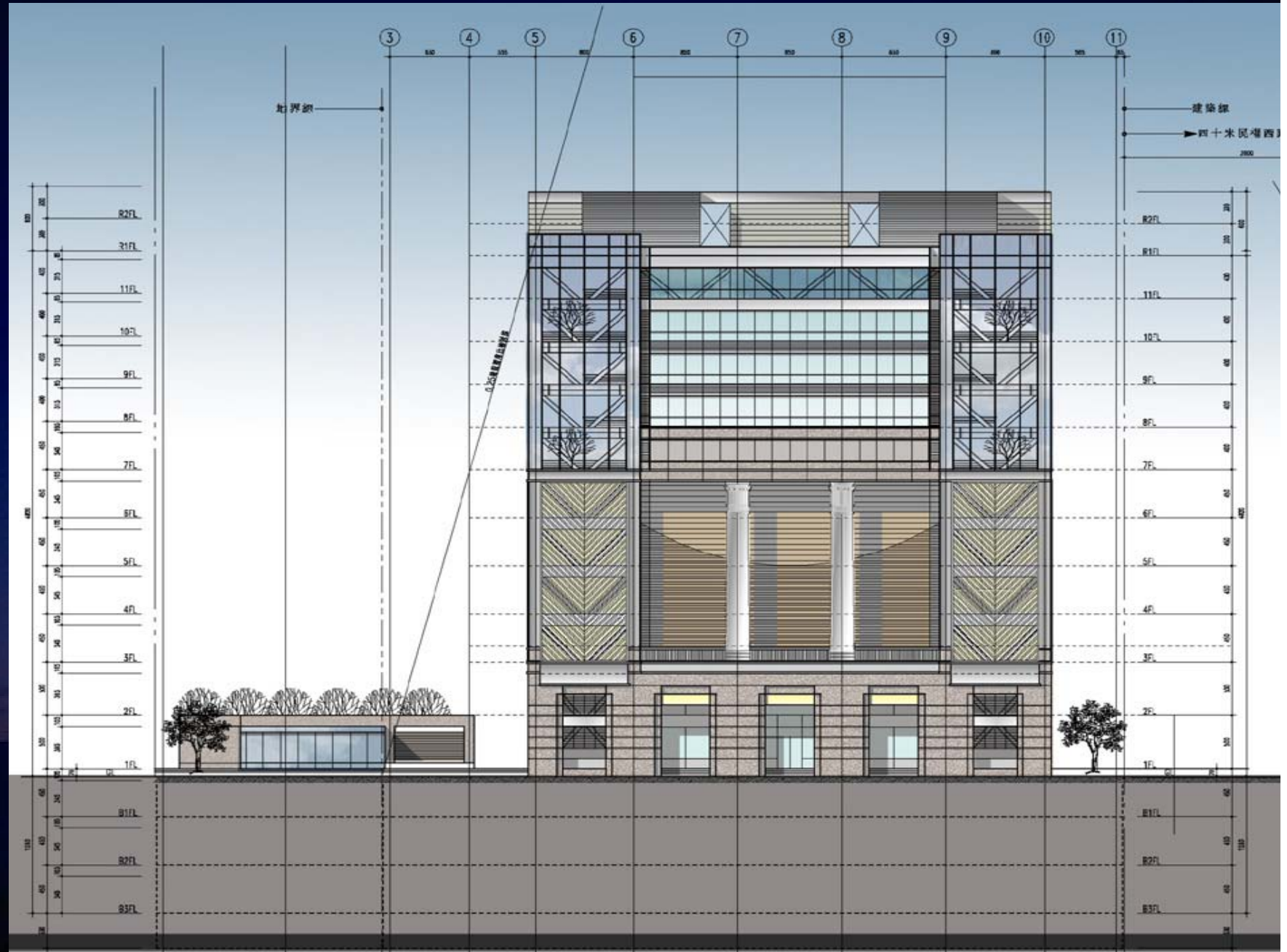
Sampling Freq. (Hz) 200

Duration Time (second) 120.000

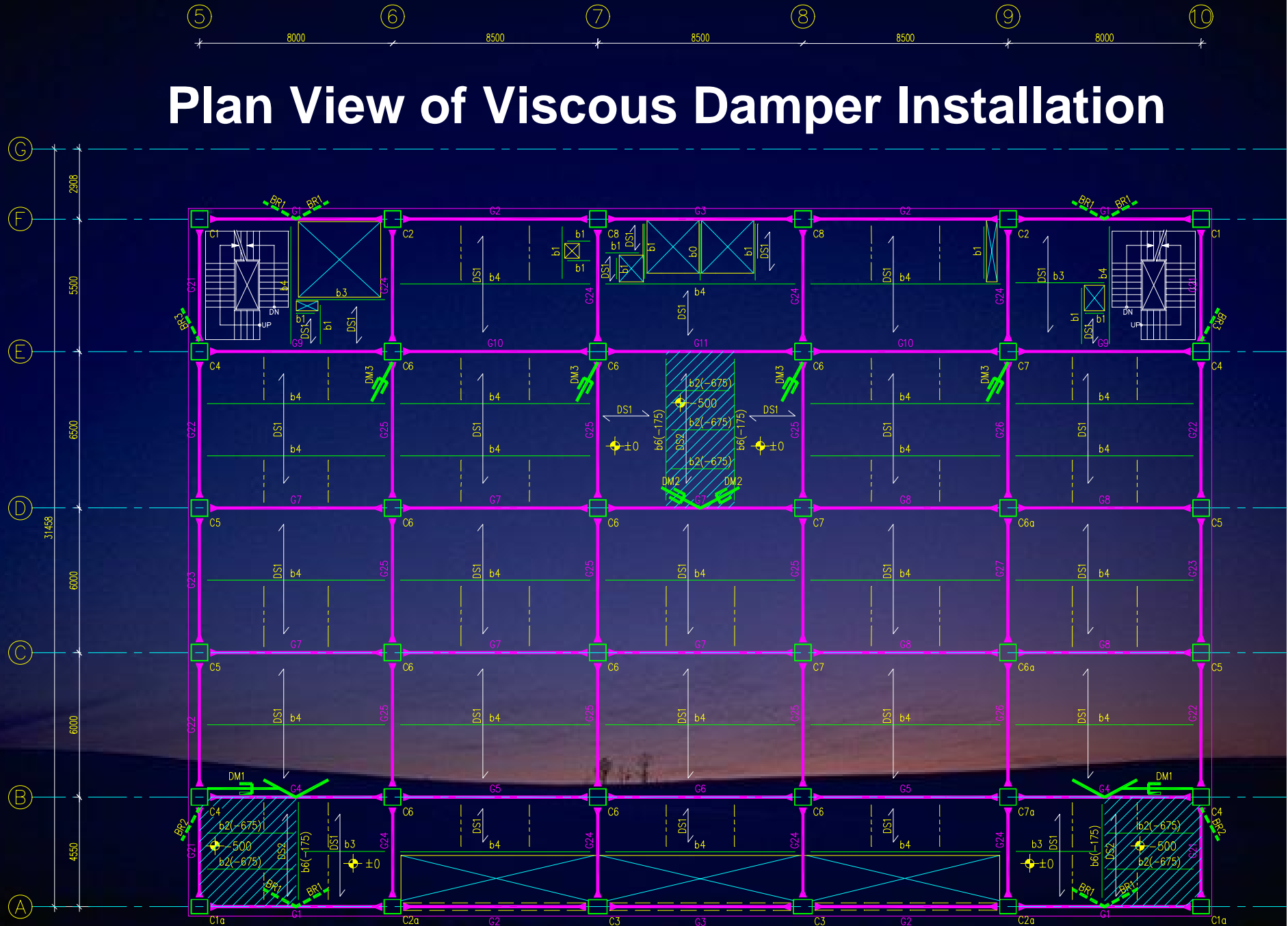
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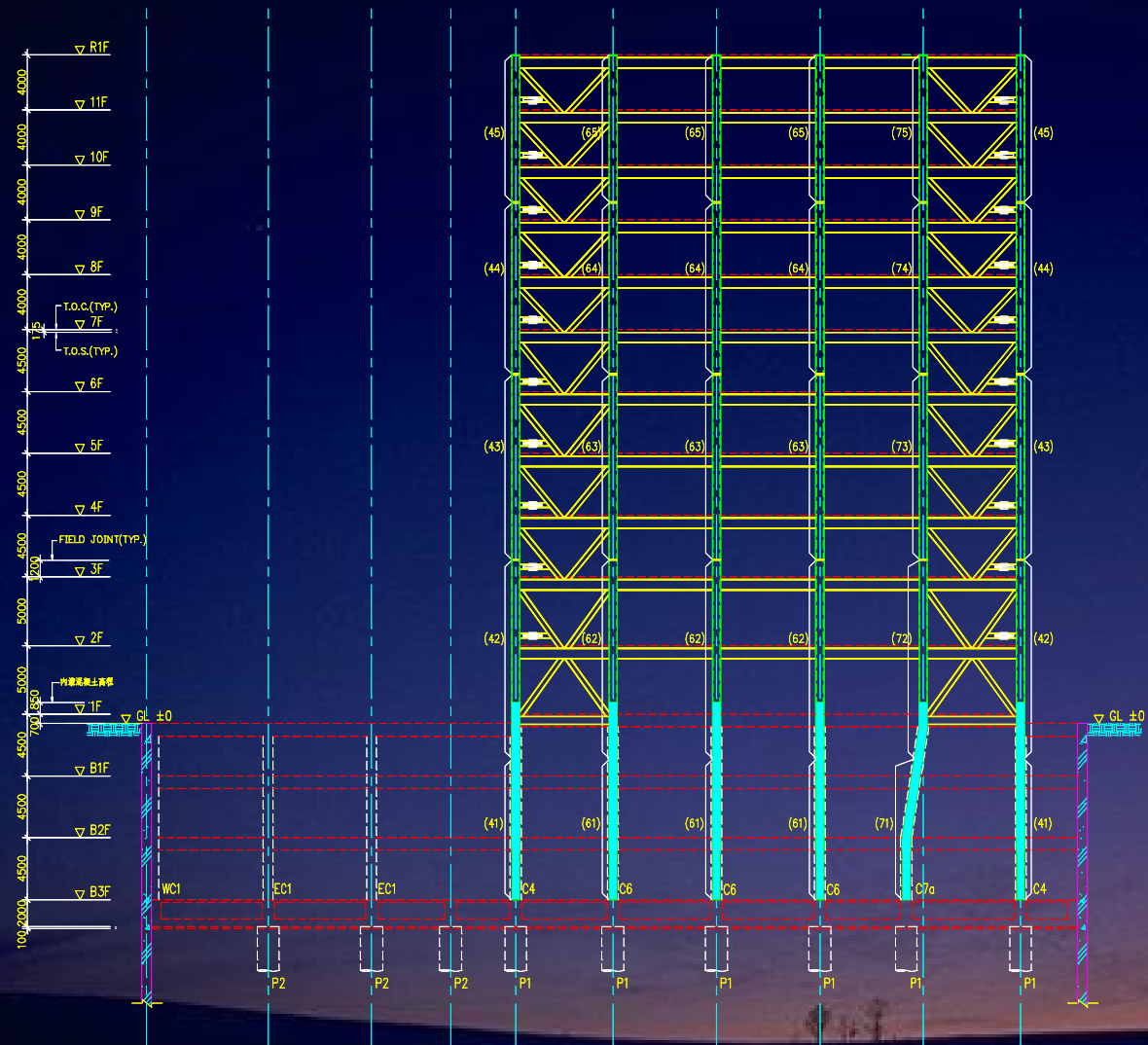


Bank of Taiwan



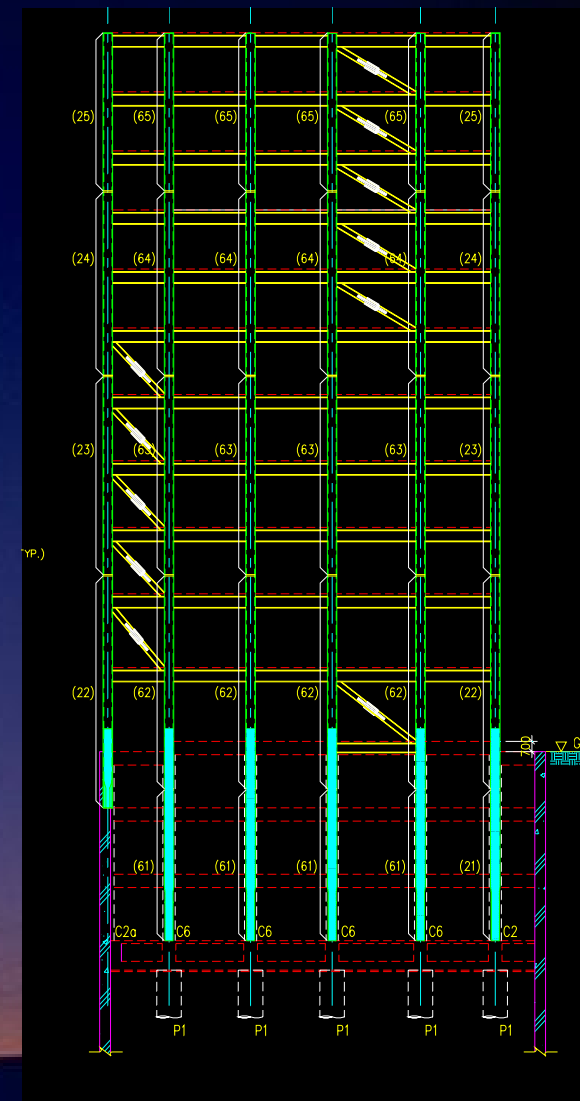
Plan View of Viscous Damper Installation





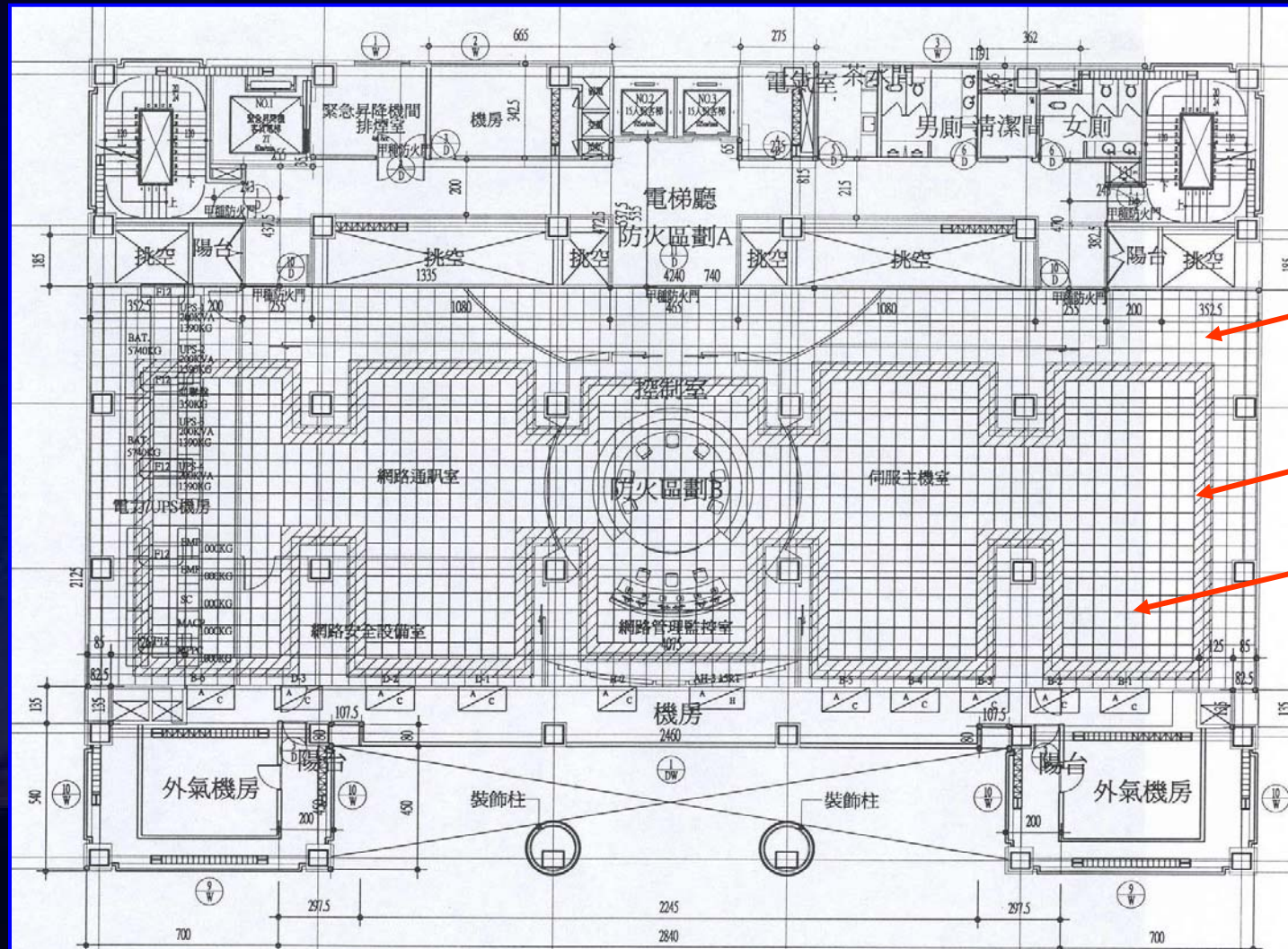
Elevation View

X-B 立構圖



Y-6 立構圖

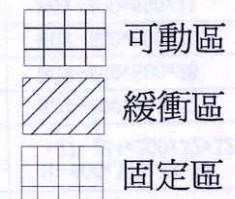
Plan View of Floor Isolation



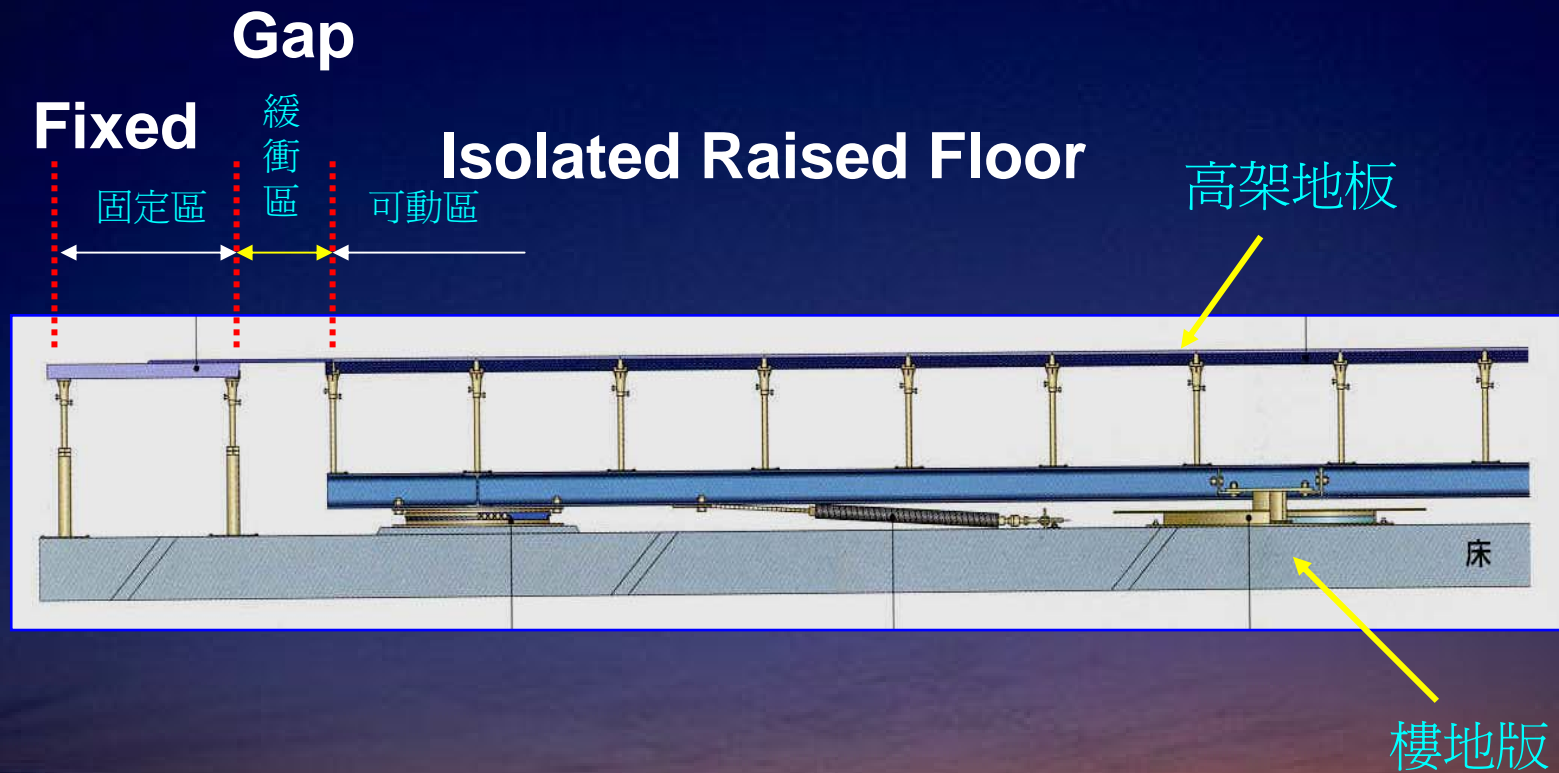
Fixed-base
raised floor

Isolation Gap

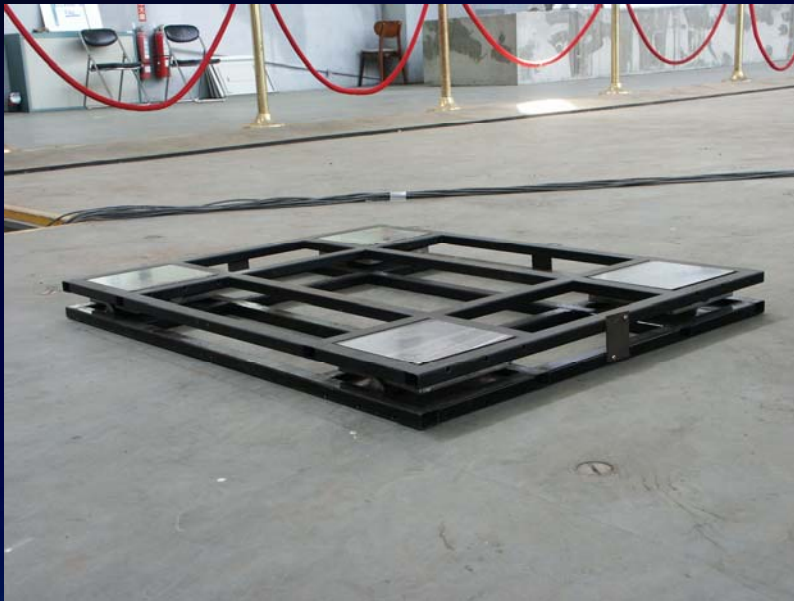
Isolated
Raised
Floor

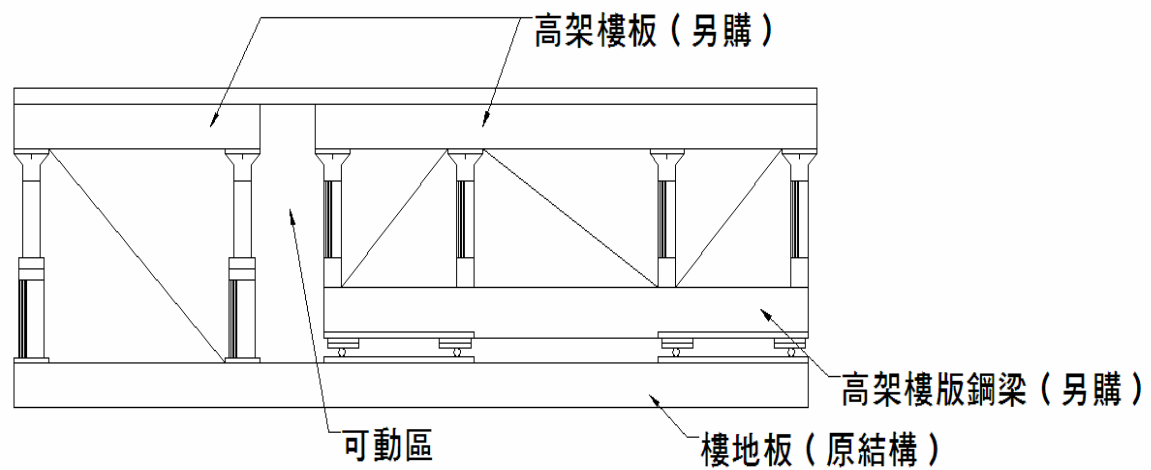


Sectional View of Floor Isolation System



A Floor Isolation System Developed by MCEER and NCREE





隔震支承與高架樓板組合示意圖



TCU XY 750gal input - X Direction Acceleration Time History Response

